SITE ENGINEERING DESIGN REPORT

Proposed Residential Subdivision
Summit Street
Derby, Connecticut
Job No. 2508

Prepared For:
Summit Hill, LLC

Prepared By:

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MARCH 5, 2025

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INTRODUCTION:

Summit Hill, LLC is proposing the construction of 36 new residential duplex houses and 8-single family detached dwellings on the two parcels totaling 17.725 acres at the end of Summit Street. The majority of the proposed dwellings will have a total floor area of approximately 2,080 square feet, distributed over two (2) stories.

Currently, the property is made up of 2 parcels. The entire site in general is a wooded area with little understory with some rock outcrops.

The stormwater runoff from the site flows in two directions southwest to Coon Hollow Brook which flows southerly and connects to an unnamed canal to the Housatonic River. The North portion of the site flows northeast towards Indian Avenue.

This site has an existing accessway to Summit Street. Two roadways are proposed for this development accessing Summit Street and Mountain Street in Derby. Upon completion of the development, garage parking will be provided for 88 cars, these spaces will be garage spaces located in the houses 2 car garages.

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EXISTING STORMWATER RUNOFF

For analysis purposes, the site has been examined as having two drainage areas. These two drainage areas will be referred to as DA-S (West & Southern drainage area flowing to Coon Hollow Brook), and DA-N (East and Northern drainage area flowing to Mountain Street).

Based on the above, it is apparent that there is two points of interest associated with the existing drainage pattern, the point immediately prior to the southwesterly portion of the property to a Coon Hollow Brook, where the site drains, and the point immediately prior to the Northeasterly portion of the property. Peak rates of stormwater runoff, for 5, 10, 25, 50 and 100-year storm events, have been calculated for this point of interest. The rates are depicted on Table 1. The supporting calculations are included as Appendix A.

These existing flows will later be compared to post-development flows as a means of assessing the potential flow impact on surrounding infrastructures.

TABLE 1

Existing Flows (CFS)

Existing Runoff from the Northeastern watershed area to be developed

DA-2

5-year	10-year	25-year	50-year	100-year
2.703	6.704	9.512	11.68	14.03

Existing Runoff from the Southwestern watershed area to be developed

DA-1

5-year	10-year	25-year	50-year	100-year
11.25	27.93	39.62	48.62	58.40

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PROPOSED STORM WATER DRAINAGE

The plans are designed to minimize potential impacts to adjacent properties and the existing stormwater infrastructure.

Without stormwater quality measures, the proposed development of this site will increase the Weighted Runoff Coefficient (WRC) and decrease the Time of Concentration (Tc), resulting in an increase in the peak rate of stormwater runoff. However, the construction of two (2) Storm Water Management Areas (SWMA) actually reduces the post-development rate of runoff relative to predevelopment conditions.

Design details for these systems are presented on Sheet SP-2 (part of the overall Project Documents). The system will drain the Roads, sidewalks, and grassy areas, associated with the 36 new residential duplex houses and 8-single-family detached homes development. The Roads will be the major elements of the total impervious area on the site. (calculations included as Appendix A, Gutter flow analysis included as Appendix B) The roof, grassy areas, sidewalks and driveways will contribute to the runoff totals seen in Table 2. The Roofs and driveways will be controlled by separate individual systems on each lot designed at a later time when the house's final layouts are known.

The southeasterly SWMA (South Pond) has a total storage volume of approximately 100,685 c. f. and the maximum water depth has been calculated to be 14.5 feet. This SWMA will reduce the post development peak rate of runoff by 2.9 cfs for the one-hundred-year storm. The northwesterly SWMA (North Galleries) has a total storage volume of approximately 14,403 c. f. and is 600 linear feet of 4x4 concrete gallery. This SWMA will reduce the post development peak rate of runoff by 1.9 cfs for the one-hundred-year storm. As indicated in the following summary and attached computations, the proposed development will reduce the peak rate of runoff to less than the pre-developed condition for both the Southwesterly and Northeasterly water sheds on site.
The development as proposed will not have an adverse impact on the surrounding wetlands or watercourses.

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Table 2, (below) represents the combined effect of these flow patterns on the existing infrastructures. Flows from the proposed systems will produce peak flows that are less than the existing peak flows from the two watersheds on the site.

TABLE 2

Proposed Flows (CFS) for new condition

Proposed Runoff from Northeastern watershed area to be developed

DA-2

2-year	10-year	25-year	50-year	100-year
1.714	5.904	7.696	8.898	12.05

Percent reduction of runoff from the site

2 year	**10 year**	**25 year**	**50year**	**100year**
36%	**11%**	**19%**	**24%**	**14%**

Proposed Runoff from Southwestern watershed area to be developed

DA-1

2-year	10-year	25-year	50-year	100-year
0.0	0.0	7.942	28.21	55.5

Percent reduction of runoff from the site

2 year	**10 year**	**25 year**	**50year**	**100year**
100%	**100%**	**80%**	**42%**	**5%**

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SANITARY SEWER

Sanitary Sewer discharge will be through a proposed 8-inch PVC sanitary sewer line to the existing sewer main in Summit Street.

Using the technical standards of the Connecticut Public Health Code, the estimated sewage flow is 150 gallons per day per bedroom. This development will have no more than 240 bedrooms :

36 lots have 6 bedrooms + 8 lots have 3 bedrooms
(36 x 6) + (8x3)= 240 total bedrooms
240 x 150 = 36,000 gallons per day average flow

Average Daily Flow = 25.0 gpm per phase

Peak flow estimate = 25.0 x 5 (peaking factor) =125

= 125 gpm Peak or 0.278 cfs

Other Utilities
All proposed utilities to the site will be through underground utility connections. Electrical service will be from existing aboveground electric utility line in proposed streets. Water service will be from an existing water main in the proposed streets.

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APPENDIX A
STAGE HYDROGRAPHS

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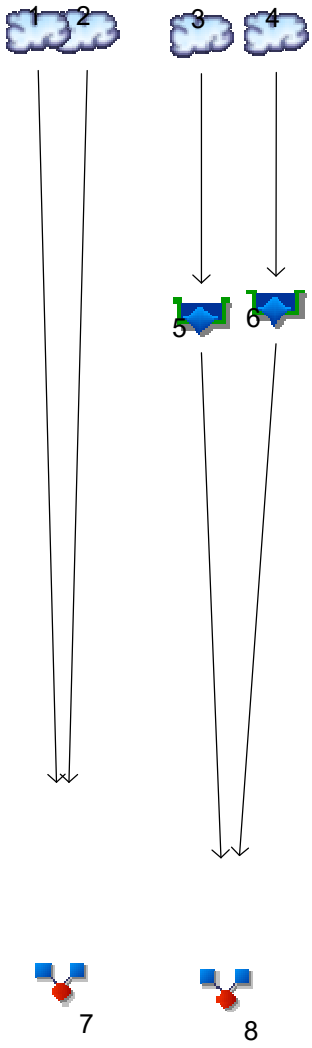
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Watershed Model Schematic



Legend

Hyd.	Origin	Description
1	SCS Runoff	EX DA-1
2	SCS Runoff	EX DA-2
3	SCS Runoff	PRO DA-1
4	SCS Runoff	PROP DA-2
5	Reservoir	FROM SOUTH POND
6	Reservoir	North Galleries
7	Combine	TOTAL EXISTING
8	Combine	TOTAL PROPOSED

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Hydrograph Return Period Recap

Hyd. No.	Hydrograph type (origin)	Inflow hyd(s)	Peak Outflow (cfs)							Hydrograph Description
			1-yr	2-yr	3-yr	5-yr	10-yr	25-yr	50-yr	100-yr	
1	SCS Runoff	------	-------	11.25	-------	-------	27.93	39.62	48.62	58.40	EX DA-1
2	SCS Runoff	------	-------	2.703	-------	-------	6.704	9.512	11.68	14.03	EX DA-2
3	SCS Runoff	------	-------	20.83	-------	-------	47.67	66.11	80.16	95.33	PRO DA-1
4	SCS Runoff	------	-------	5.658	-------	-------	12.68	17.45	21.08	24.99	PROP DA-2
5	Reservoir	3	-------	0.000	-------	-------	0.000	7.942	28.21	55.50	FROM SOUTH POND
6	Reservoir	4	-------	1.714	-------	-------	5.904	7.696	8.898	12.05	North Galleries
7	Combine	1, 2,	-------	13.92	-------	-------	34.54	48.98	60.13	72.27	TOTAL EXISTING
8	Combine	5, 6,	-------	1.714	-------	-------	5.904	13.00	36.89	67.54	TOTAL PROPOSED

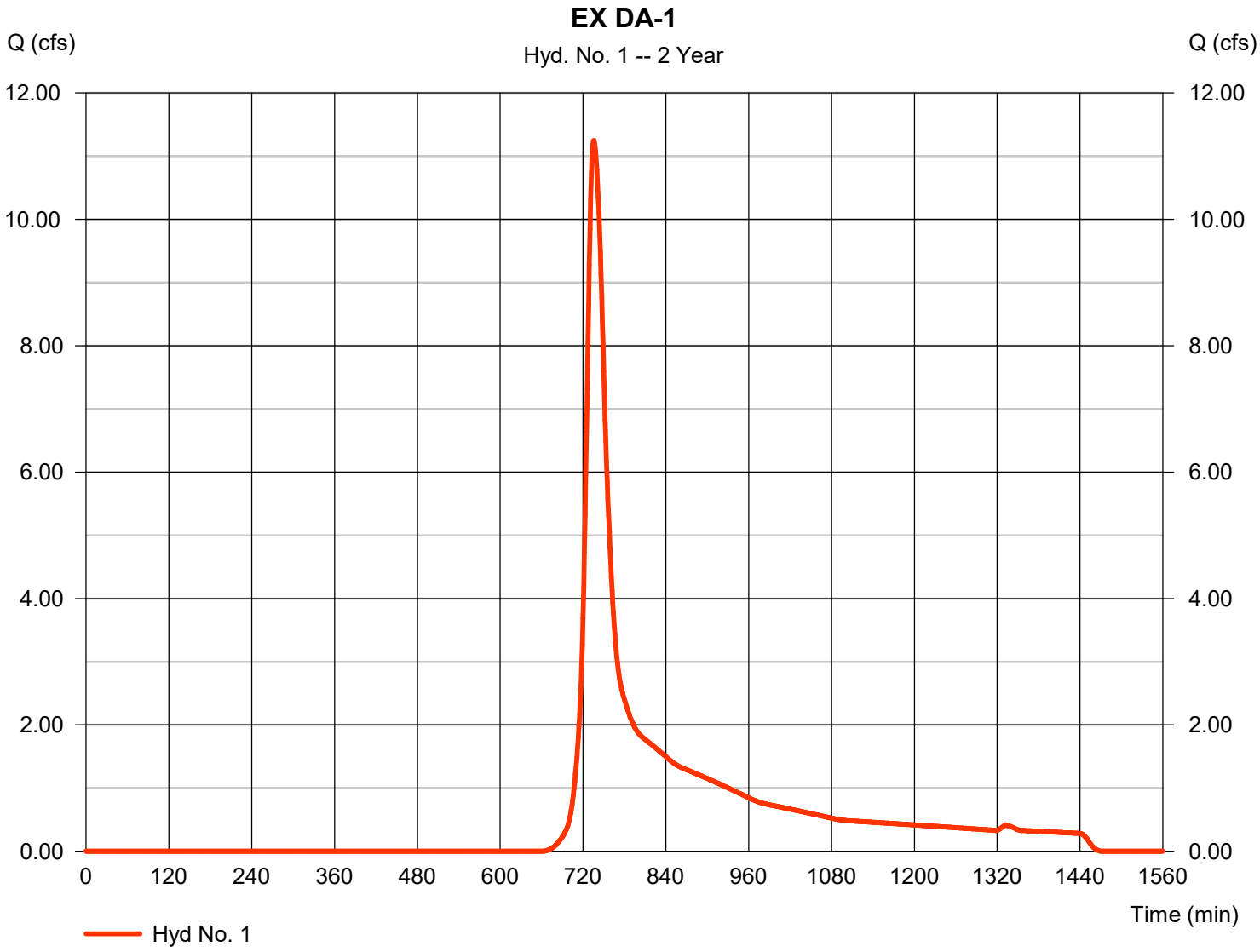
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Hydrograph Report

Hyd. No. 1

EX DA-1

Hydrograph type	=	SCS Runoff	Peak discharge	=	11.25 cfs
Storm frequency	=	2 yrs	Time to peak	=	736 min
Time interval	=	1 min	Hyd. volume	=	53,956 cuft
Drainage area	=	14.140 ac	Curve number	=	69
Basin Slope	=	0.0 %	Hydraulic length	=	0 ft
Tc method	=	TR55	Time of conc. (Tc)	=	18.80 min
Total precip.	=	3.66 in	Distribution	=	Type III
Storm duration	=	24 hrs	Shape factor	=	484



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TR55 Tc Worksheet

Hyd. No. 1

EX DA-1

Description	A		B		C		Totals
Sheet Flow							
Manning's n-value	= 0.400		0.011		0.011		
Flow length (ft)	= 300.0		0.0		0.0		
Two-year 24-hr precip. (in)	= 3.66		0.00		0.00		
Land slope (%)	= 23.00		0.00		0.00		
Travel Time (min)	**= 18.20**	**+**	**0.00**	**+**	**0.00**	**=**	**18.20**
Shallow Concentrated Flow							
Flow length (ft)	= 280.00		0.00		0.00		
Watercourse slope (%)	= 23.00		0.00		0.00		
Surface description	= Unpaved		Paved		Paved		
Average velocity (ft/s)	=7.74		0.00		0.00		
Travel Time (min)	**= 0.60**	**+**	**0.00**	**+**	**0.00**	**=**	**0.60**
Channel Flow							
X sectional flow area (sqft)	= 0.00		0.00		0.00		
Wetted perimeter (ft)	= 0.00		0.00		0.00		
Channel slope (%)	= 0.00		0.00		0.00		
Manning's n-value	= 0.015		0.015		0.015		
Velocity (ft/s)	=0.00						
			0.00				
					0.00		
Flow length (ft)	({0})0.0		0.0		0.0		
Travel Time (min)	**= 0.00**	**+**	**0.00**	**+**	**0.00**	**=**	**0.00**

Total Travel Time, Tc ..... 18.80 min

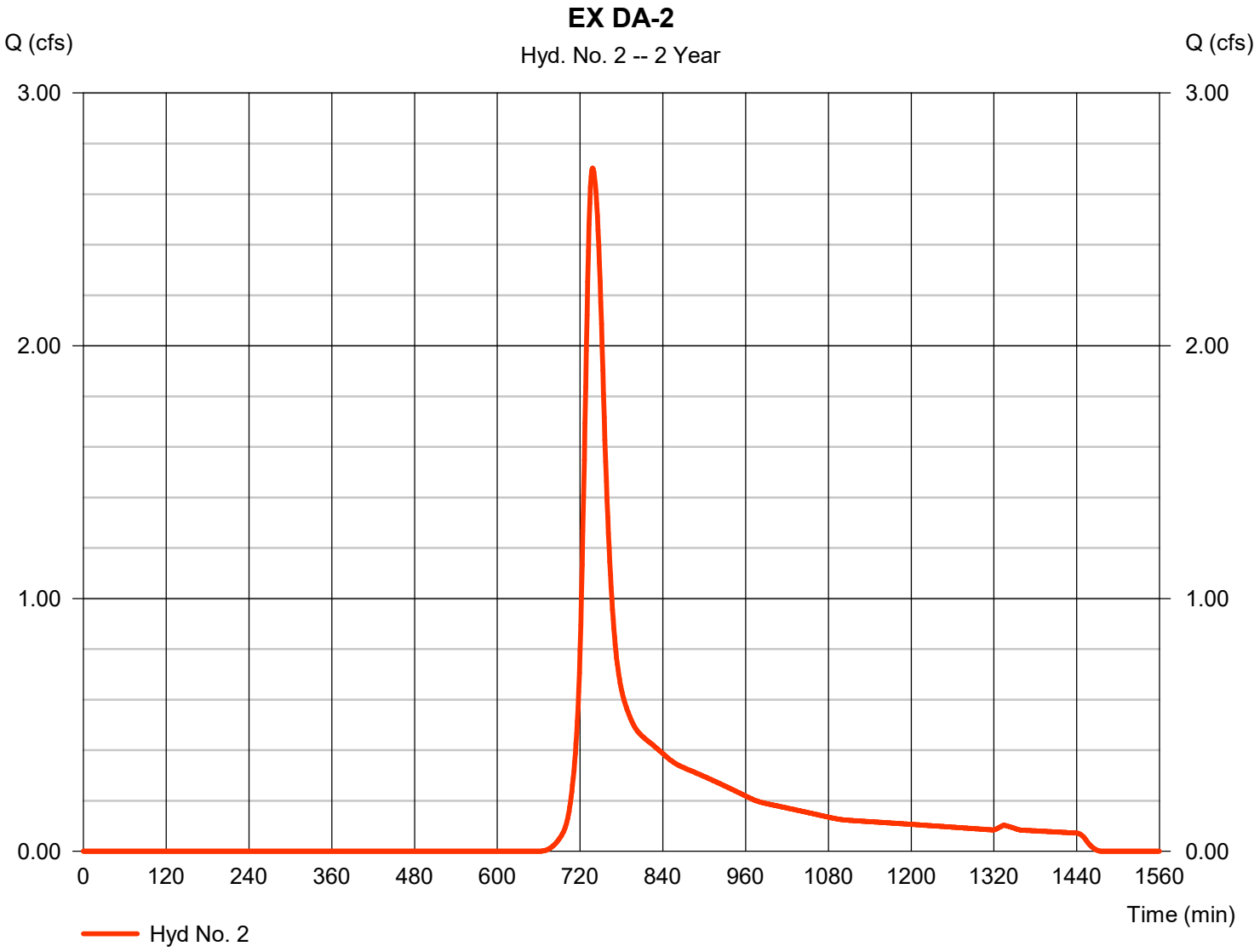
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Hydrograph Report

Hyd. No. 2

EX DA-2

Hydrograph type	=	SCS Runoff	Peak discharge	=	2.703 cfs
Storm frequency	=	2 yrs	Time to peak	=	738 min
Time interval	=	1 min	Hyd. volume	=	13,728 cuft
Drainage area	=	3.630 ac	Curve number	=	69
Basin Slope	=	0.0 %	Hydraulic length	=	0 ft
Tc method	=	TR55	Time of conc. (Tc)	=	22.70 min
Total precip.	=	3.66 in	Distribution	=	Type III
Storm duration	=	24 hrs	Shape factor	=	484



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TR55 Tc Worksheet

Hyd. No. 2

EX DA-2

Description	A		B		C		Totals
Sheet Flow							
Manning's n-value	= 0.400		0.011		0.011		
Flow length (ft)	= 300.0		0.0		0.0		
Two-year 24-hr precip. (in)	= 3.66		0.00		0.00		
Land slope (%)	= 14.00		0.00		0.00		
Travel Time (min)	**= 22.20**	**+**	**0.00**	**+**	**0.00**	**=**	**22.20**
Shallow Concentrated Flow							
Flow length (ft)	= 190.00		0.00		0.00		
Watercourse slope (%)	= 14.00		0.00		0.00		
Surface description	= Unpaved		Paved		Paved		
Average velocity (ft/s)	=6.04		0.00		0.00		
Travel Time (min)	**= 0.52**	**+**	**0.00**	**+**	**0.00**	**=**	**0.52**
Channel Flow							
X sectional flow area (sqft)	= 0.00		0.00		0.00		
Wetted perimeter (ft)	= 0.00		0.00		0.00		
Channel slope (%)	= 0.00		0.00		0.00		
Manning's n-value	= 0.015		0.015		0.015		
Velocity (ft/s)	=0.00						
			0.00				
					0.00		
Flow length (ft)	({0})0.0		0.0		0.0		
Travel Time (min)	**= 0.00**	**+**	**0.00**	**+**	**0.00**	**=**	**0.00**

Total Travel Time, Tc ..... 22.70 min

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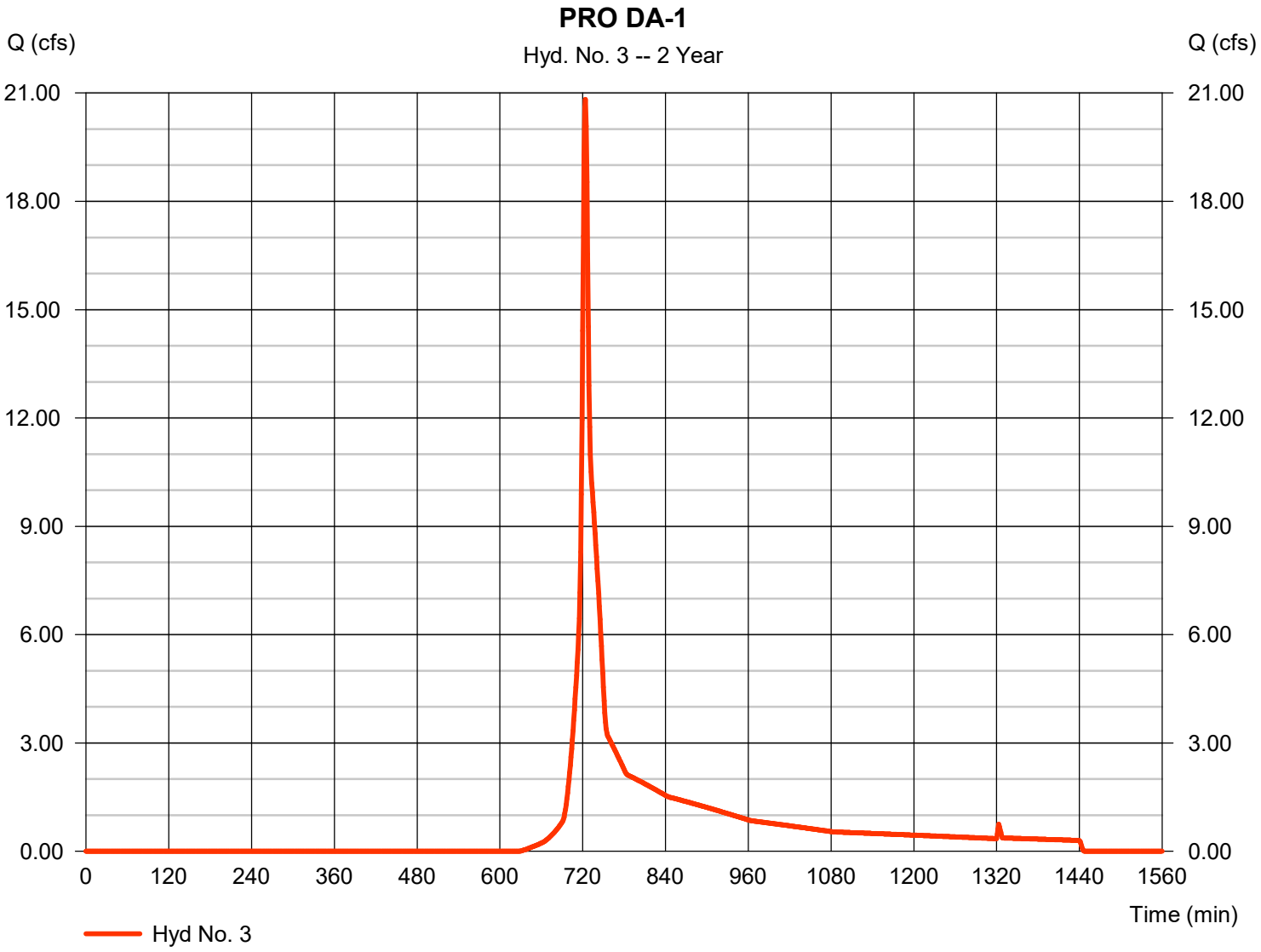
Hydrograph Report

Hyd. No. 3

PRO DA-1

Hydrograph type	=	SCS Runoff	Peak discharge	=	20.83 cfs
Storm frequency	=	2 yrs	Time to peak	=	724 min
Time interval	=	1 min	Hyd. volume	=	62,973 cuft
Drainage area	=	14.140 ac	Curve number	=	72*
Basin Slope	=	0.0 %	Hydraulic length	=	0 ft
Tc method	=	TR55	Time of conc. (Tc)	=	4.00 min
Total precip.	=	3.66 in	Distribution	=	Type III
Storm duration	=	24 hrs	Shape factor	=	484

* Composite (Area/CN) = [(1.660 x 98) + (12.480 x 69)] / 14.140



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TR55 Tc Worksheet

Hyd. No. 3

PRO DA-1

Description	A		B		C		Totals
Sheet Flow							
Manning's n-value	= 0.150		0.011		0.011		
Flow length (ft)	= 40.0		24.0		0.0		
Two-year 24-hr precip. (in)	= 3.66		3.66		0.00		
Land slope (%)	= 8.00		10.00		0.00		
Travel Time (min)	**= 2.53**	**+**	**0.19**	**+**	**0.00**	**=**	**2.72**
Shallow Concentrated Flow							
Flow length (ft)	= 0.00		0.00		0.00		
Watercourse slope (%)	= 0.00		0.00		0.00		
Surface description	= Paved		Paved		Paved		
Average velocity (ft/s)	=0.00		0.00		0.00		
Travel Time (min)	**= 0.00**	**+**	**0.00**	**+**	**0.00**	**=**	**0.00**
Channel Flow							
X sectional flow area (sqft)	= 1.23		0.00		0.00		
Wetted perimeter (ft)	= 3.93		0.00		0.00		
Channel slope (%)	= 8.00		0.00		0.00		
Manning's n-value	= 0.011		0.015		0.015		
Velocity (ft/s)	=17.59						
			0.00				
					0.00		
Flow length (ft)	({0})1320.0		0.0		0.0		
Travel Time (min)	**= 1.25**	**+**	**0.00**	**+**	**0.00**	**=**	**1.25**

Total Travel Time, Tc ..... 4.00 min

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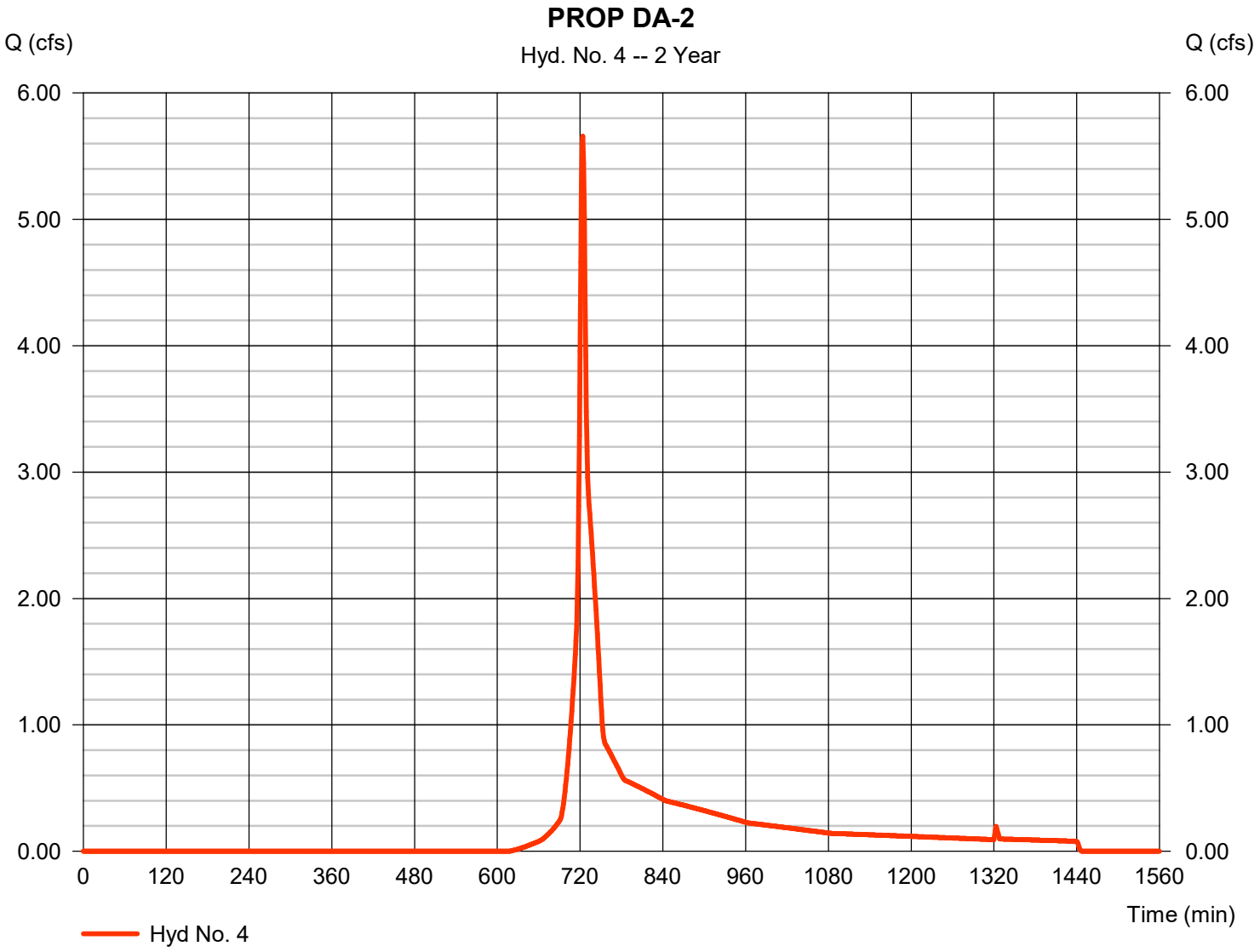
Hydrograph Report

Hyd. No. 4

PROP DA-2

Hydrograph type	=	SCS Runoff	Peak discharge	=	5.658 cfs
Storm frequency	=	2 yrs	Time to peak	=	724 min
Time interval	=	1 min	Hyd. volume	=	16,978 cuft
Drainage area	=	3.630 ac	Curve number	=	73*
Basin Slope	=	0.0 %	Hydraulic length	=	0 ft
Tc method	=	TR55	Time of conc. (Tc)	=	3.90 min
Total precip.	=	3.66 in	Distribution	=	Type III
Storm duration	=	24 hrs	Shape factor	=	484

* Composite (Area/CN) = [(0.471 x 98) + (3.160 x 69)] / 3.630



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TR55 Tc Worksheet

Hyd. No. 4

PROP DA-2

Description	A		B		C		Totals
Sheet Flow							
Manning's n-value	= 0.150		0.011		0.011		
Flow length (ft)	= 40.0		164.0		0.0		
Two-year 24-hr precip. (in)	= 3.66		3.66		0.00		
Land slope (%)	= 8.00		6.50		0.00		
Travel Time (min)	**= 2.53**	**+**	**1.05**	**+**	**0.00**	**=**	**3.58**
Shallow Concentrated Flow							
Flow length (ft)	= 0.00		0.00		0.00		
Watercourse slope (%)	= 0.00		0.00		0.00		
Surface description	= Paved		Paved		Paved		
Average velocity (ft/s)	=0.00		0.00		0.00		
Travel Time (min)	**= 0.00**	**+**	**0.00**	**+**	**0.00**	**=**	**0.00**
Channel Flow							
X sectional flow area (sqft)	= 1.23		0.00		0.00		
Wetted perimeter (ft)	= 3.93		0.00		0.00		
Channel slope (%)	= 6.20		0.00		0.00		
Manning's n-value	= 0.011		0.015		0.015		
Velocity (ft/s)	=15.49						
			0.00				
					0.00		
Flow length (ft)	({0})310.0		0.0		0.0		
Travel Time (min)	**= 0.33**	**+**	**0.00**	**+**	**0.00**	**=**	**0.33**

Total Travel Time, Tc ..... 3.90 min

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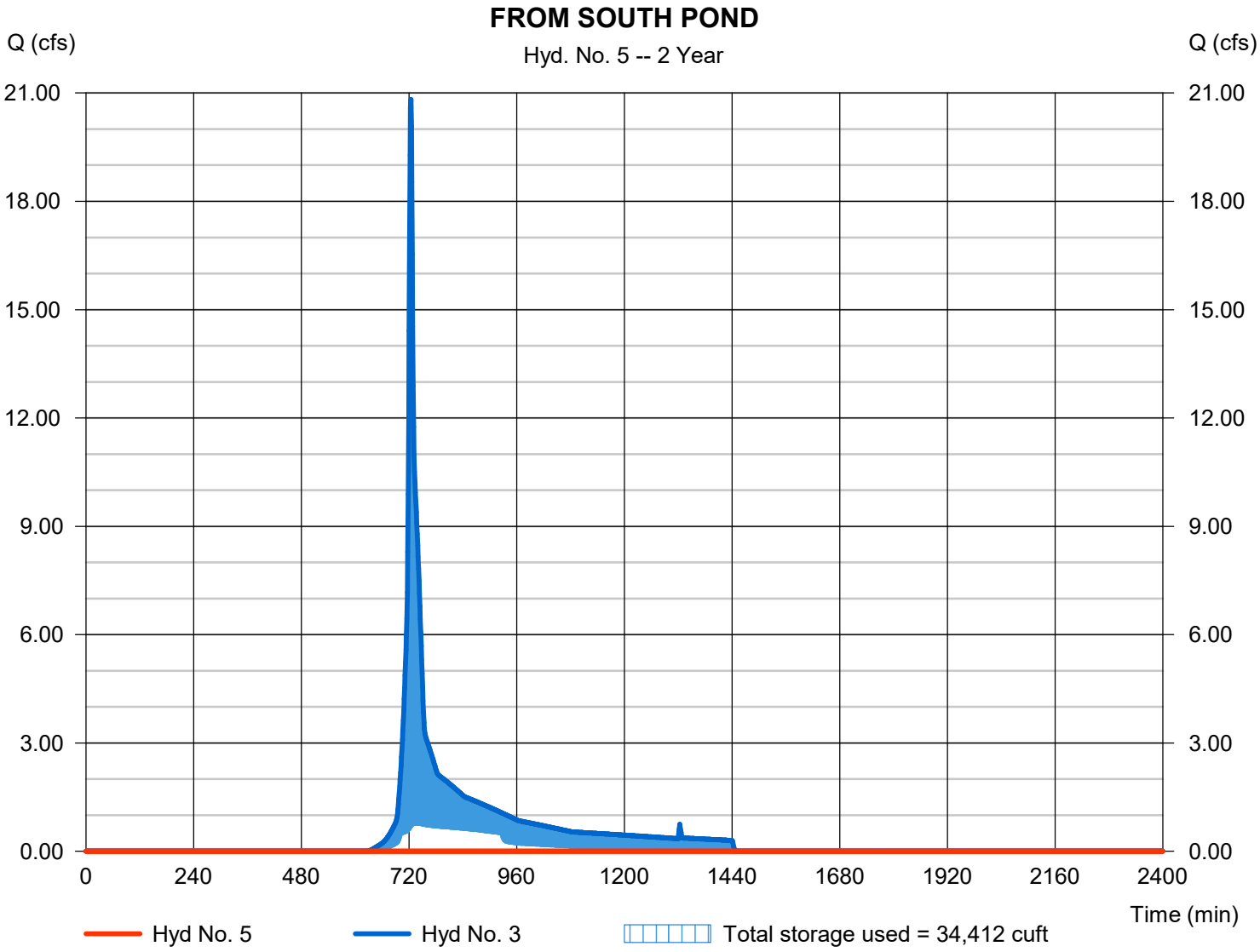
Hydrograph Report

Hyd. No. 5

FROM SOUTH POND

Hydrograph type	=	Reservoir	Peak discharge	=	0.000 cfs
Storm frequency	=	2 yrs	Time to peak	=	2141 min
Time interval	=	1 min	Hyd. volume	=	0 cuft
Inflow hyd. No.	=	3 - PRO DA-1	Max. Elevation	=	169.45 ft
Reservoir name	=	SOUTH POND	Max. Storage	=	34,412 cuft

Storage Indication method used. Exfiltration extracted from Outflow.



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Pond Report

Pond No. 1 - SOUTH POND

Pond Data

Contours -User-defined contour areas. Conic method used for volume calculation. Begining Elevation = 161.50 ft

Stage / Storage Table

Stage (ft)	Elevation (ft)	Contour area (sqft)	Incr. Storage (cuft)	Total storage (cuft)
0.00	161.50	1,056	0	0
0.50	162.00	2,050	763	763
2.50	164.00	3,227	5,232	5,995
4.50	166.00	4,575	7,762	13,757
6.50	168.00	6,094	10,632	24,389
8.50	170.00	7,782	13,840	38,229
10.50	172.00	9,641	17,388	55,617
12.50	174.00	11,251	20,869	76,486
14.50	176.00	12,970	24,198	100,685

Culvert / Orifice Structures

	[A]	[B]	[C]	[PrfRsr]
Rise (in)	= 0.00	Inactive	Inactive	0.00
Span (in)	= 0.00	16.00	18.00	0.00
No. Barrels	= 0	1	1	0
Invert El. (ft)	= 0.00	162.00	165.00	0.00
Length (ft)	= 0.00	0.50	0.50	0.00
Slope (%)	= 0.00	0.00	0.00	n/a
N-Value	= .013	.013	.013	n/a
Orifice Coeff.	= 0.60	0.60	0.60	0.60
Multi-Stage	= n/a	No	No	No

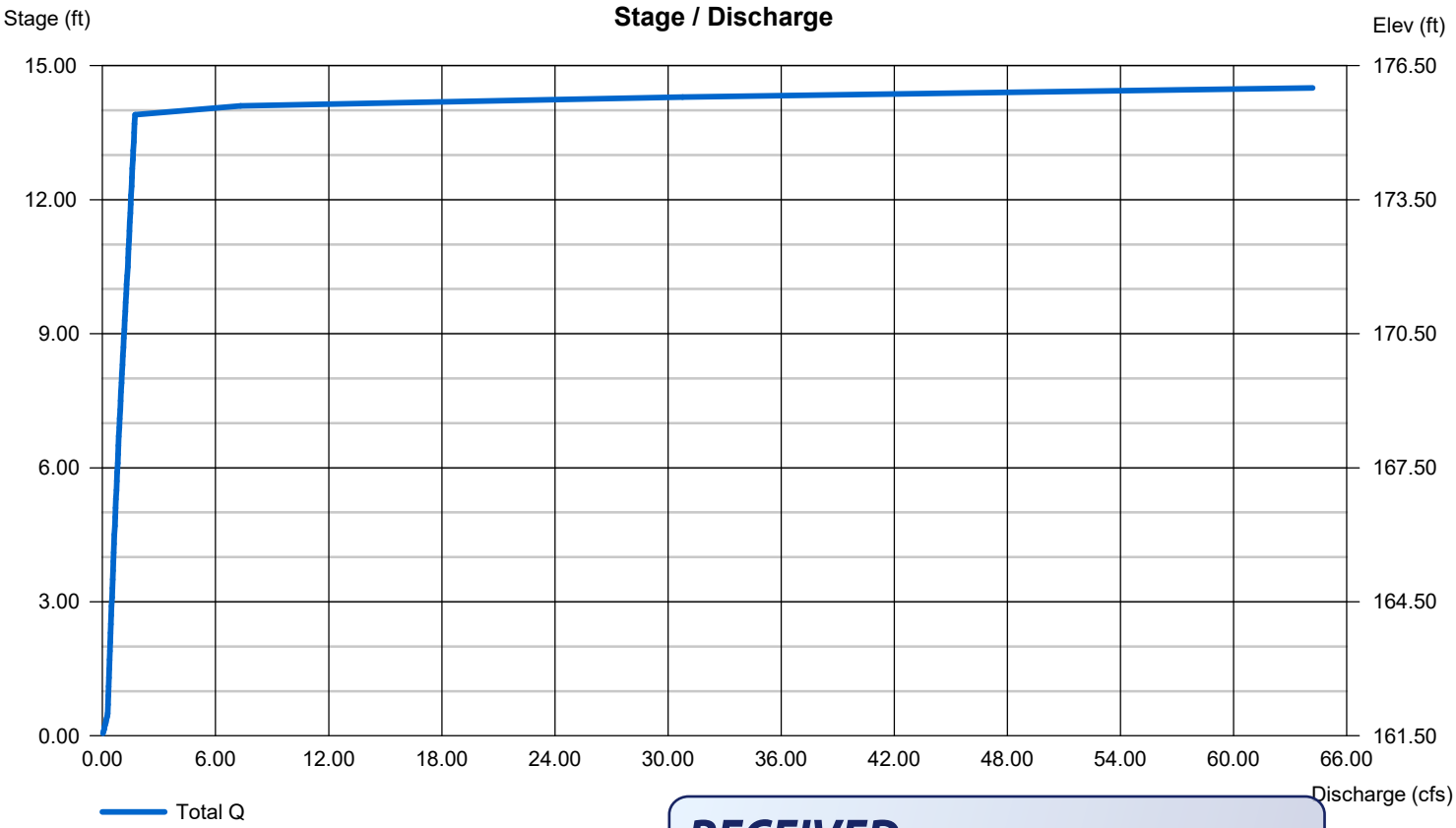
Weir Structures

	[A]	[B]	[C]	[D]
Crest Len (ft)	= 53.00	0.00	0.00	0.00
Crest El. (ft)	= 175.50	0.00	0.00	0.00
Weir Coeff.	= 3.33	3.33	3.33	3.33
Weir Type	= Rect	---	---	---
Multi-Stage	= No	No	No	No

Exfil.(in/hr) = 6.000 (by Contour)

TW Elev. (ft) = 0.00

Note: Culvert/Orifice outflows are analyzed under inlet (ic) and outlet (oc) control. Weir risers checked for orifice conditions (ic) and submergence (s).



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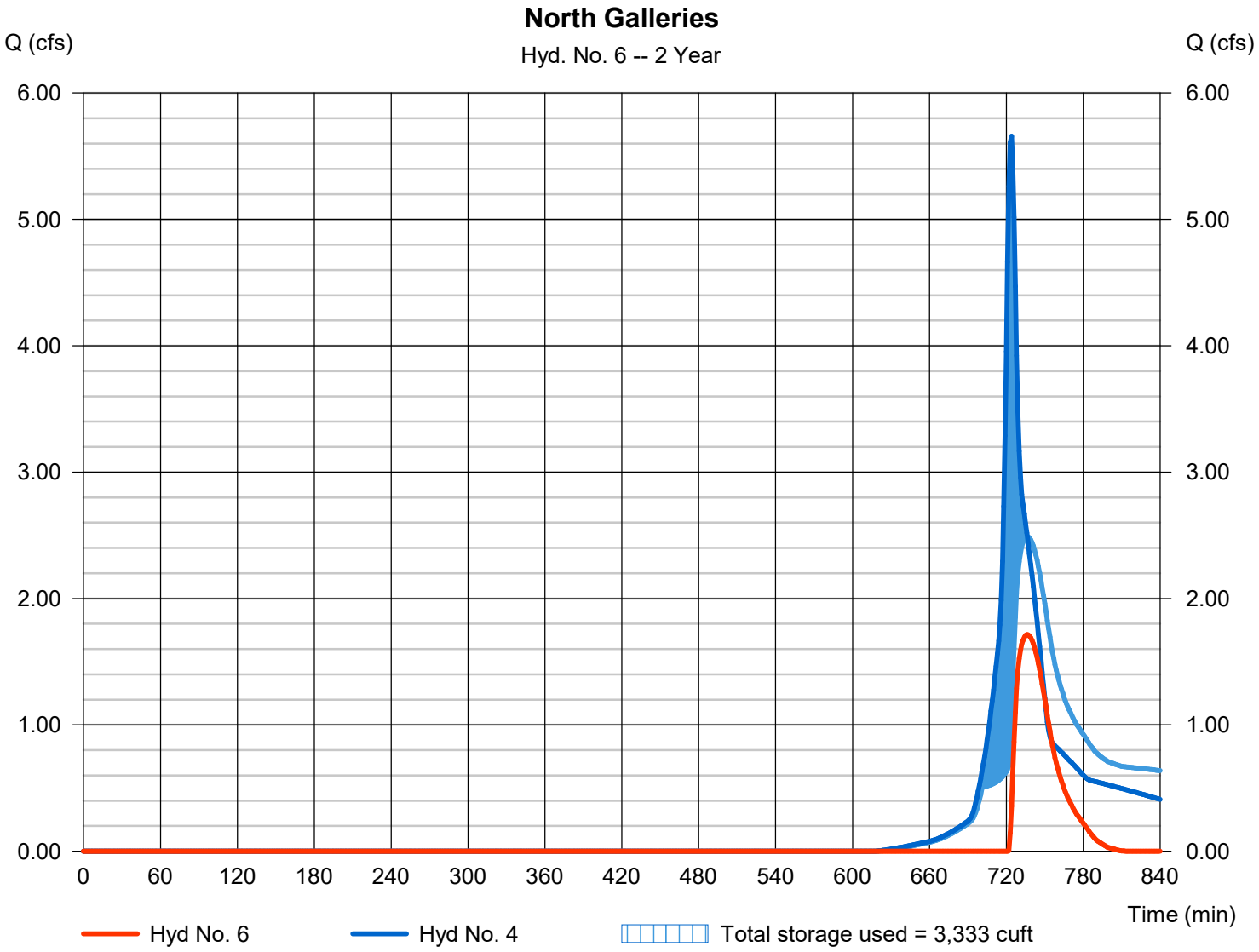
Hydrograph Report

Hyd. No. 6

North Galleries

Hydrograph type	=	Reservoir	Peak discharge	=	1.714 cfs
Storm frequency	=	2 yrs	Time to peak	=	736 min
Time interval	=	1 min	Hyd. volume	=	3,438 cuft
Inflow hyd. No.	=	4 - PROP DA-2	Max. Elevation	=	225.66 ft
Reservoir name	=	North Galleries	Max. Storage	=	3,333 cuft

Storage Indication method used. Exfiltration extracted from Outflow.



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Pond Report

Pond No. 2 - North Galleries

Pond Data

UG Chambers -Invert elev. = 225.00 ft, Rise x Span = 4.00 x 4.00 ft, Barrel Len = 600.00 ft, No. Barrels = 1, Slope = 0.00%, Headers = No
Encasement -Invert elev. = 224.00 ft, Width = 6.00 ft, Height = 6.00 ft, Voids = 40.00%

Stage / Storage Table

Stage (ft)	Elevation (ft)	Contour area (sqft)	Incr. Storage (cuft)	Total storage (cuft)
0.00	224.00	n/a	0	0
0.60	224.60	n/a	864	864
1.20	225.20	n/a	1,152	2,016
1.80	225.80	n/a	1,728	3,745
2.40	226.40	n/a	1,728	5,473
3.00	227.00	n/a	1,728	7,201
3.60	227.60	n/a	1,728	8,930
4.20	228.20	n/a	1,728	10,658
4.80	228.80	n/a	1,728	12,386
5.40	229.40	n/a	1,152	13,539
6.00	230.00	n/a	864	14,403

Culvert / Orifice Structures

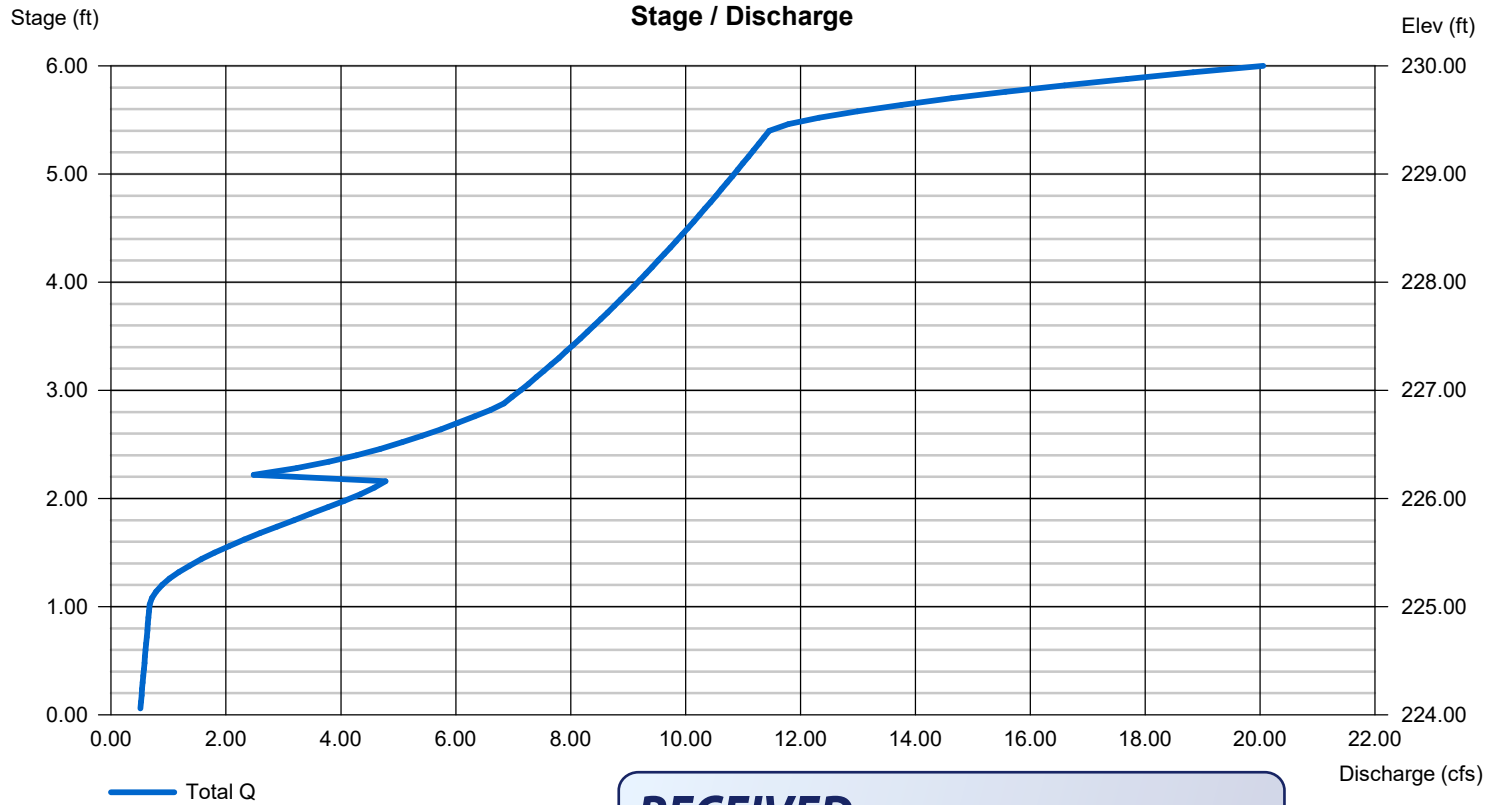
	[A]	[B]	[C]	[PrfRsr]
Rise (in)	= 0.00	14.00	0.00	0.00
Span (in)	= 0.00	14.00	0.00	0.00
No. Barrels	= 1	1	0	0
Invert El. (ft)	= 0.00	225.00	0.00	0.00
Length (ft)	= 0.00	0.50	0.00	0.00
Slope (%)	= 0.00	0.00	0.00	n/a
N-Value	= .013	.013	.013	n/a
Orifice Coeff.	= 0.60	0.60	0.60	0.60
Multi-Stage	= n/a	No	No	No

Weir Structures

	[A]	[B]	[C]	[D]
Crest Len (ft)	= 0.00	5.00	0.00	0.00
Crest El. (ft)	= 0.00	229.40	0.00	0.00
Weir Coeff.	= 3.33	3.33	3.33	3.33
Weir Type	= ---	Rect	---	---
Multi-Stage	= No	No	No	No

Exfil.(in/hr) = 6.000 (by Wet area)
TW Elev. (ft) = 0.00

Note: Culvert/Orifice outflows are analyzed under inlet (ic) and outlet (oc) control. Weir risers checked for orifice conditions (ic) and submergence (s).



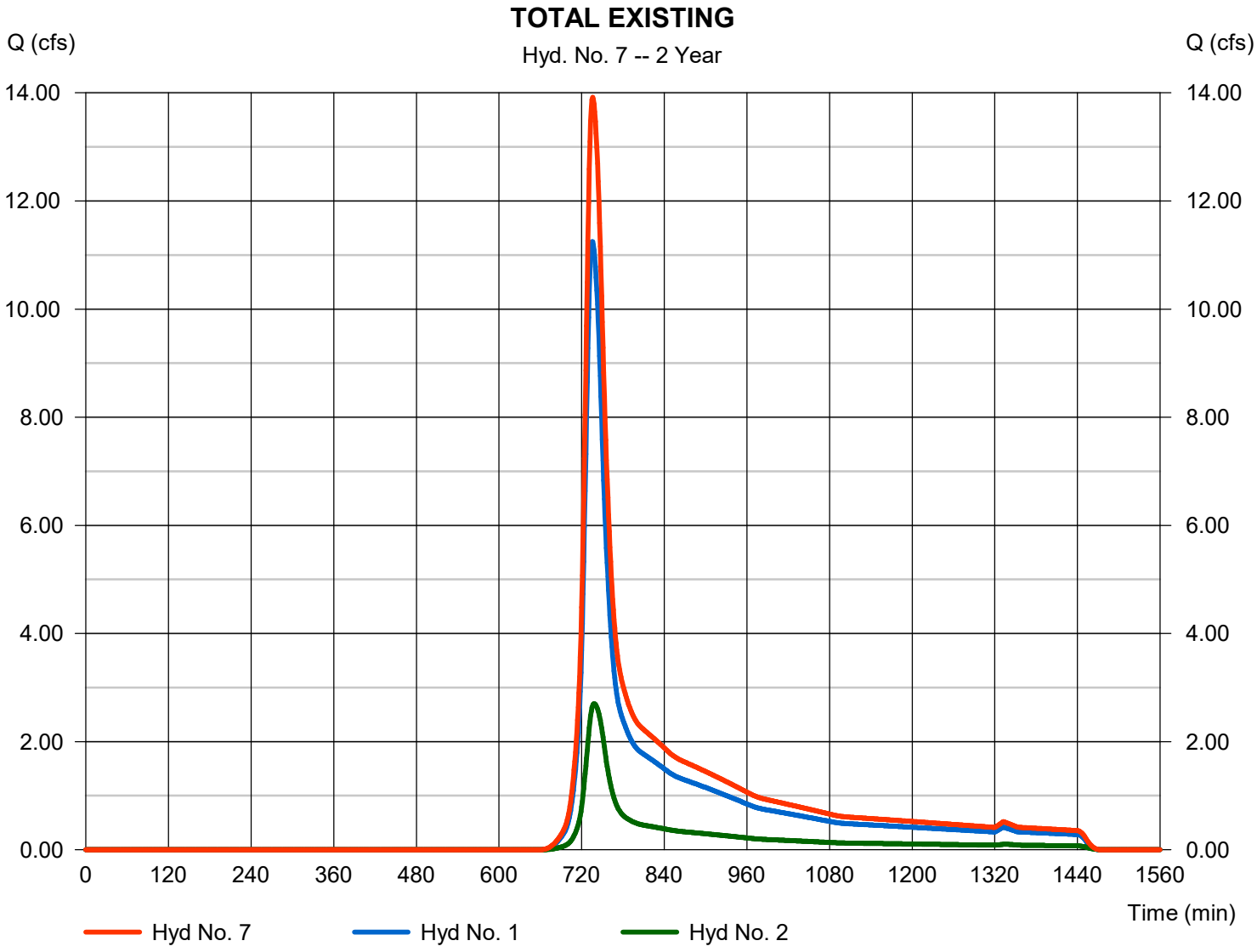
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Hydrograph Report

Hyd. No. 7

TOTAL EXISTING

Hydrograph type	= Combine	Peak discharge	= 13.92 cfs
Storm frequency	= 2 yrs	Time to peak	= 736 min
Time interval	= 1 min	Hyd. volume	= 67,684 cuft
Inflow hyds.	= 1, 2	Contrib. drain. area	= 17.770 ac



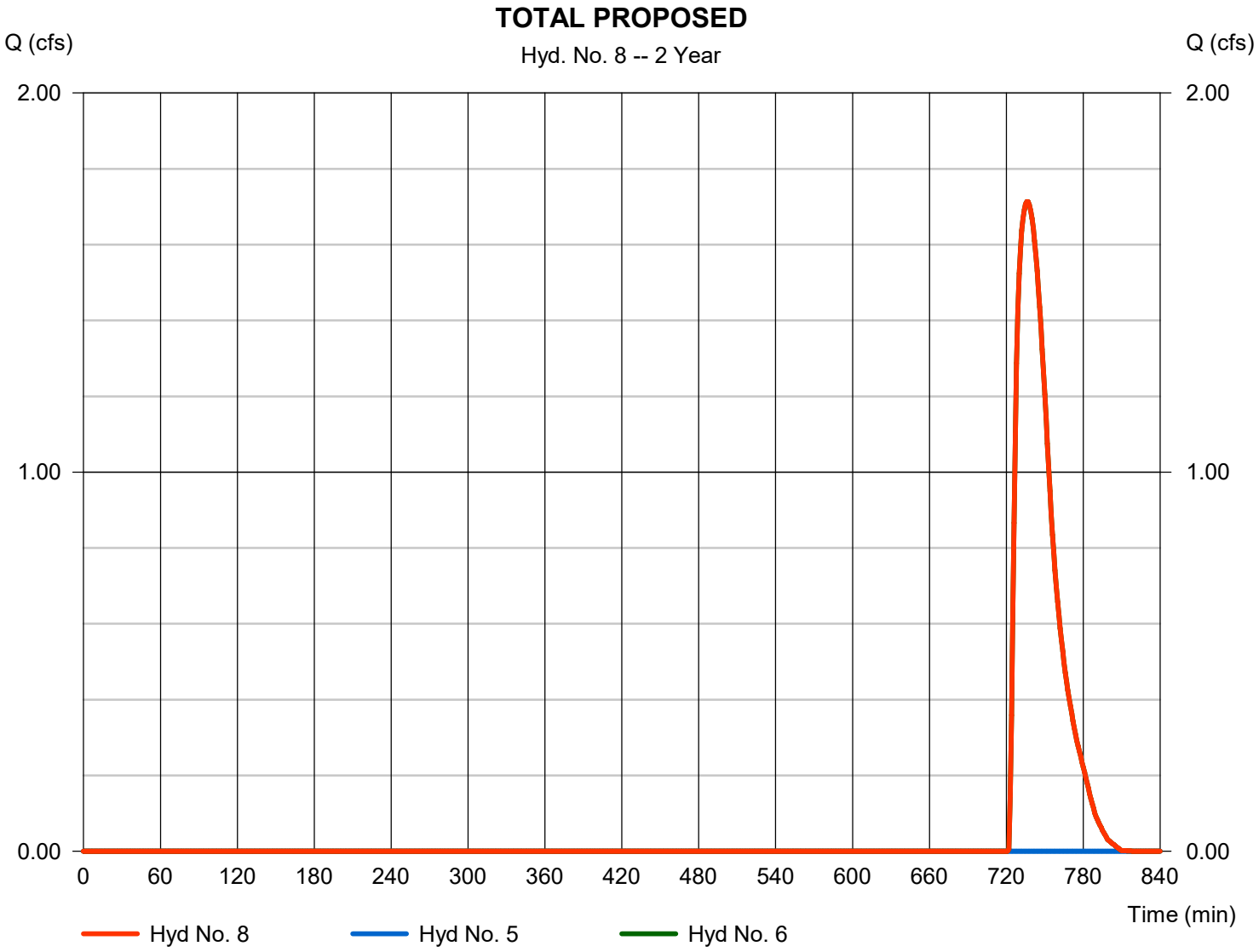
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Hydrograph Report

Hyd. No. 8

TOTAL PROPOSED

Hydrograph type	= Combine	Peak discharge	= 1.714 cfs
Storm frequency	= 2 yrs	Time to peak	= 736 min
Time interval	= 1 min	Hyd. volume	= 3,438 cuft
Inflow hyds.	= 5, 6	Contrib. drain. area	= 0.000 ac



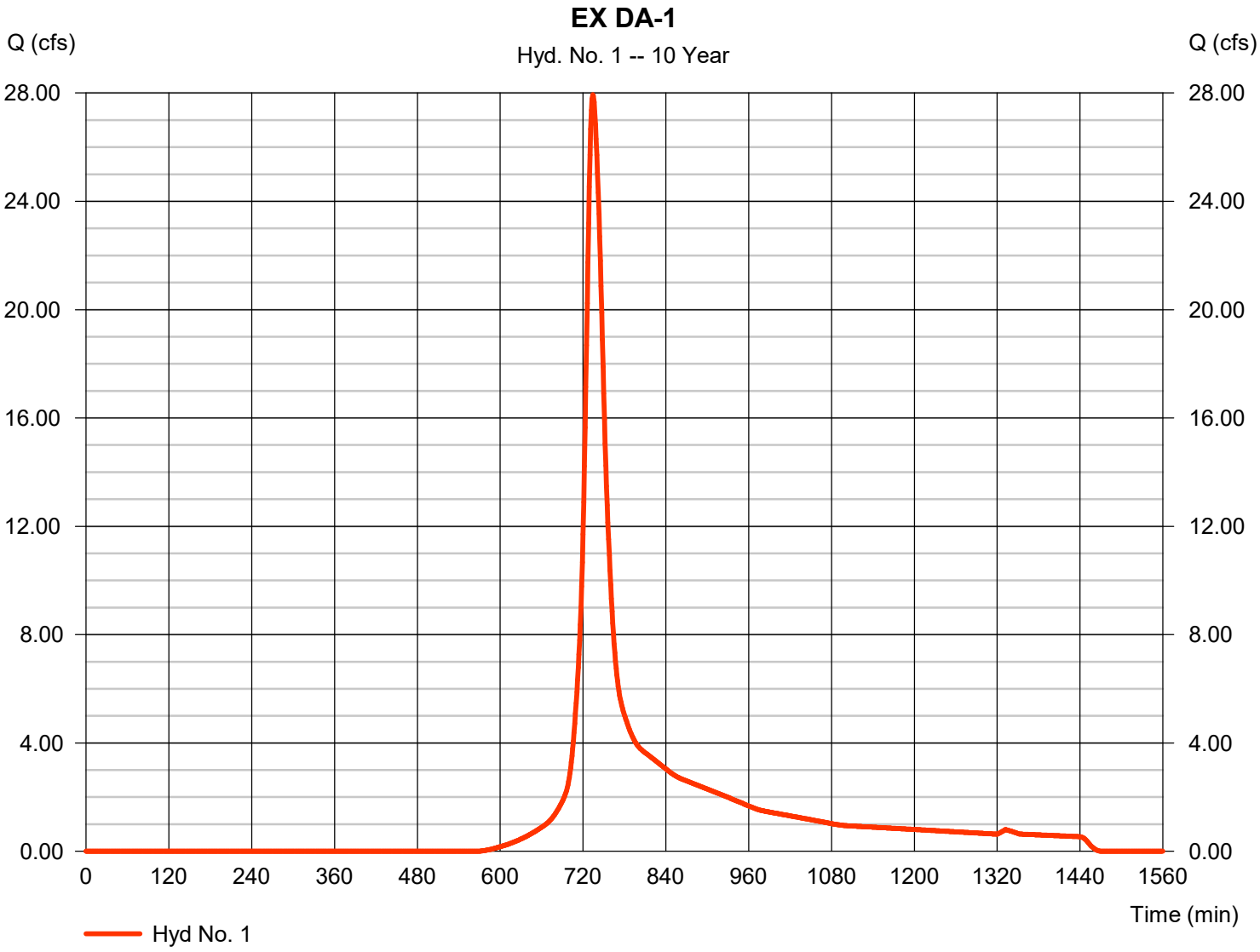
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Hydrograph Report

Hyd. No. 1

EX DA-1

Hydrograph type	=	SCS Runoff	Peak discharge	=	27.93 cfs
Storm frequency	=	10 yrs	Time to peak	=	734 min
Time interval	=	1 min	Hyd. volume	=	124,962 cuft
Drainage area	=	14.140 ac	Curve number	=	69
Basin Slope	=	0.0 %	Hydraulic length	=	0 ft
Tc method	=	TR55	Time of conc. (Tc)	=	18.80 min
Total precip.	=	5.64 in	Distribution	=	Type III
Storm duration	=	24 hrs	Shape factor	=	484



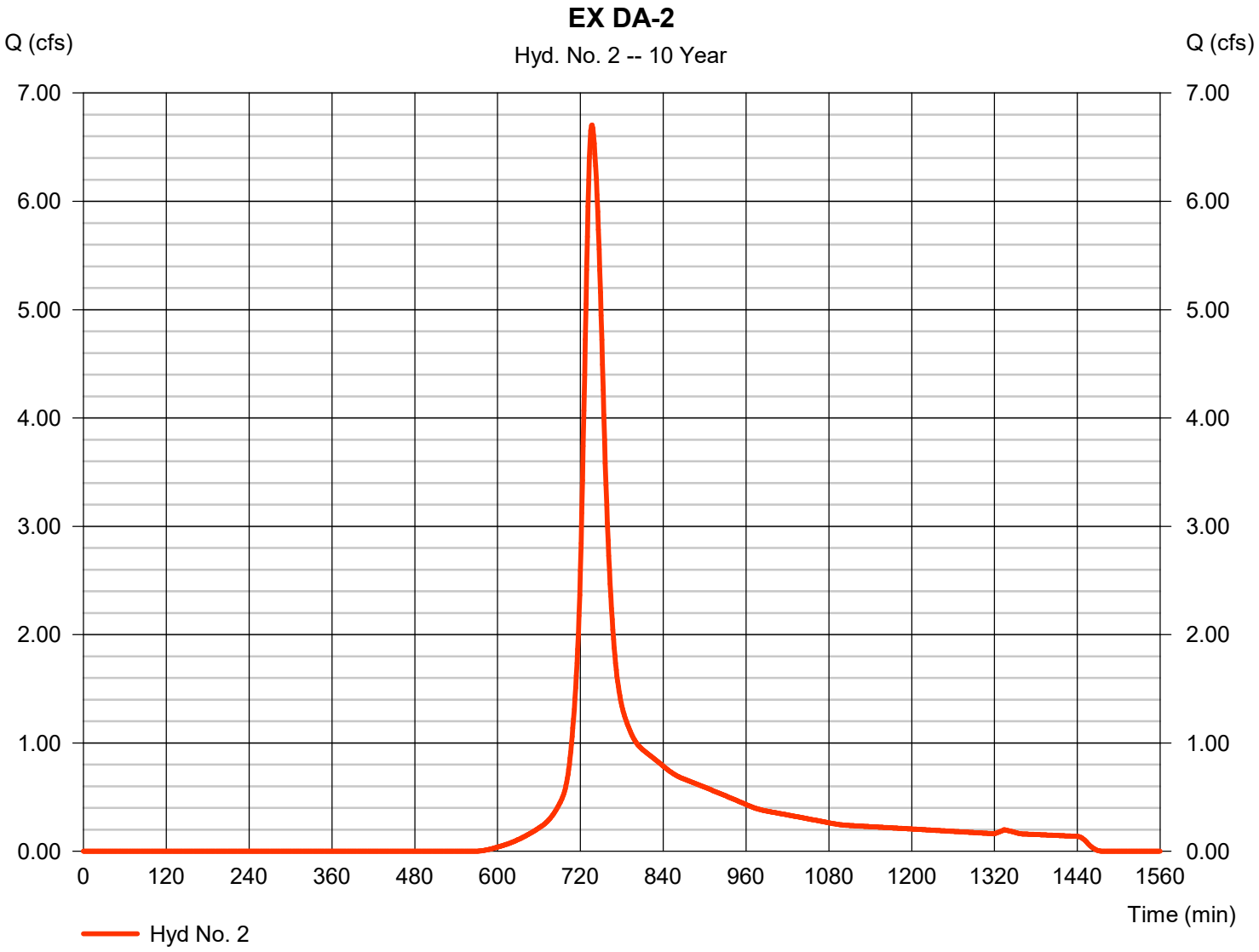
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Hydrograph Report

Hyd. No. 2

EX DA-2

Hydrograph type	=	SCS Runoff	Peak discharge	=	6.704 cfs
Storm frequency	=	10 yrs	Time to peak	=	737 min
Time interval	=	1 min	Hyd. volume	=	31,794 cuft
Drainage area	=	3.630 ac	Curve number	=	69
Basin Slope	=	0.0 %	Hydraulic length	=	0 ft
Tc method	=	TR55	Time of conc. (Tc)	=	22.70 min
Total precip.	=	5.64 in	Distribution	=	Type III
Storm duration	=	24 hrs	Shape factor	=	484



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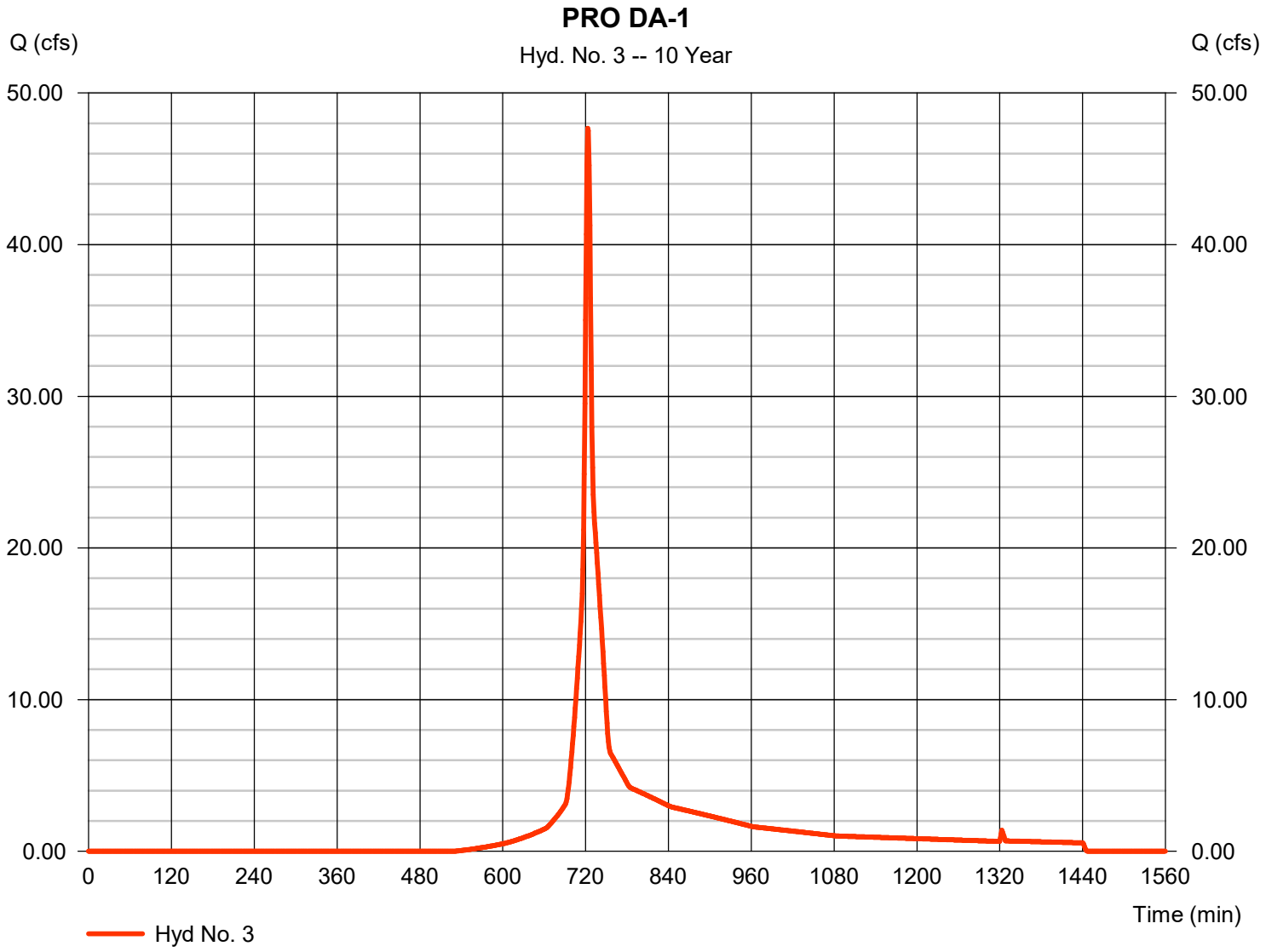
Hydrograph Report

Hyd. No. 3

PRO DA-1

Hydrograph type	=	SCS Runoff	Peak discharge	=	47.67 cfs
Storm frequency	=	10 yrs	Time to peak	=	723 min
Time interval	=	1 min	Hyd. volume	=	138,663 cuft
Drainage area	=	14.140 ac	Curve number	=	72*
Basin Slope	=	0.0 %	Hydraulic length	=	0 ft
Tc method	=	TR55	Time of conc. (Tc)	=	4.00 min
Total precip.	=	5.64 in	Distribution	=	Type III
Storm duration	=	24 hrs	Shape factor	=	484

* Composite (Area/CN) = [(1.660 x 98) + (12.480 x 69)] / 14.140



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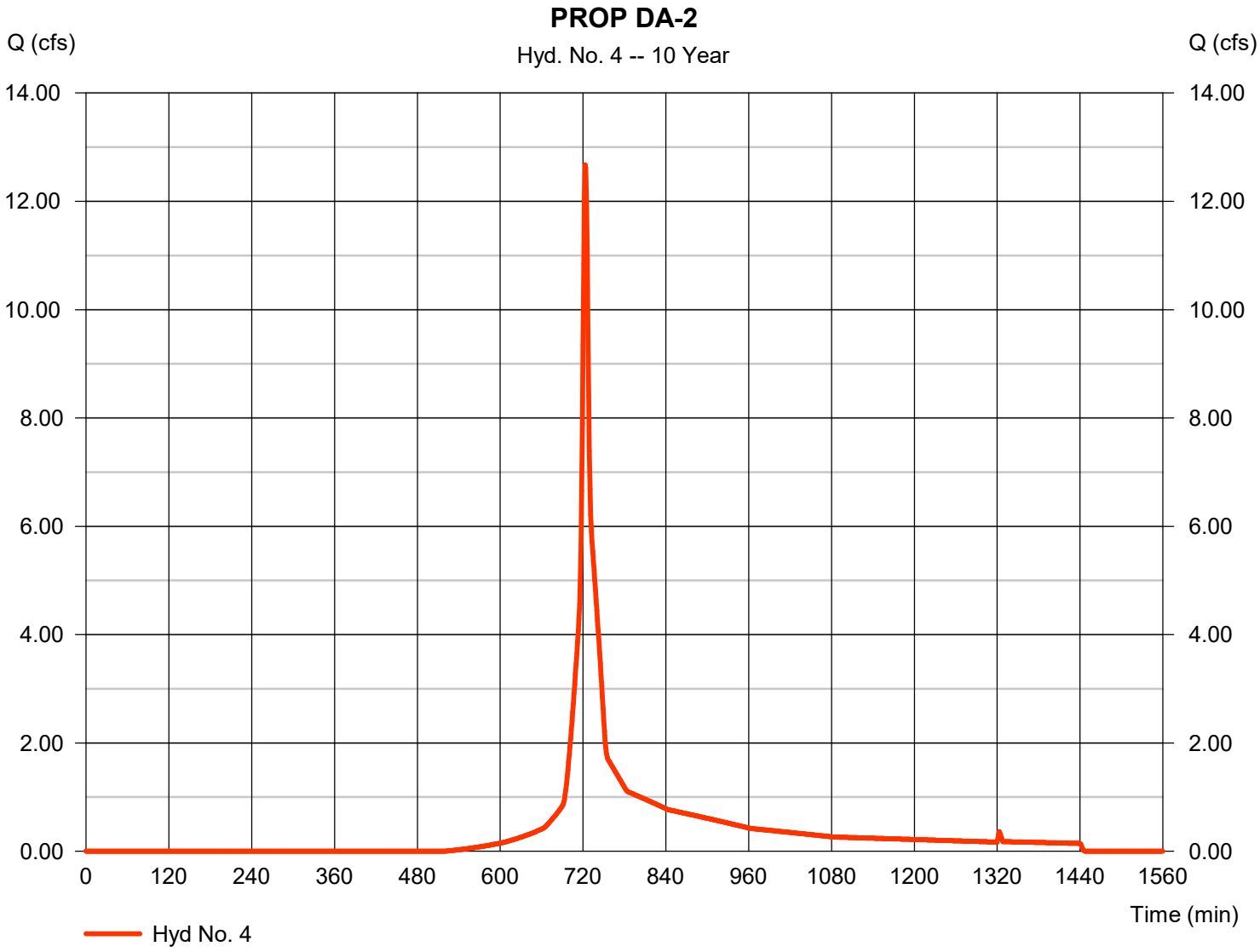
Hydrograph Report

Hyd. No. 4

PROP DA-2

Hydrograph type	=	SCS Runoff	Peak discharge	=	12.68 cfs
Storm frequency	=	10 yrs	Time to peak	=	723 min
Time interval	=	1 min	Hyd. volume	=	36,797 cuft
Drainage area	=	3.630 ac	Curve number	=	73*
Basin Slope	=	0.0 %	Hydraulic length	=	0 ft
Tc method	=	TR55	Time of conc. (Tc)	=	3.90 min
Total precip.	=	5.64 in	Distribution	=	Type III
Storm duration	=	24 hrs	Shape factor	=	484

* Composite (Area/CN) = [(0.471 x 98) + (3.160 x 69)] / 3.630



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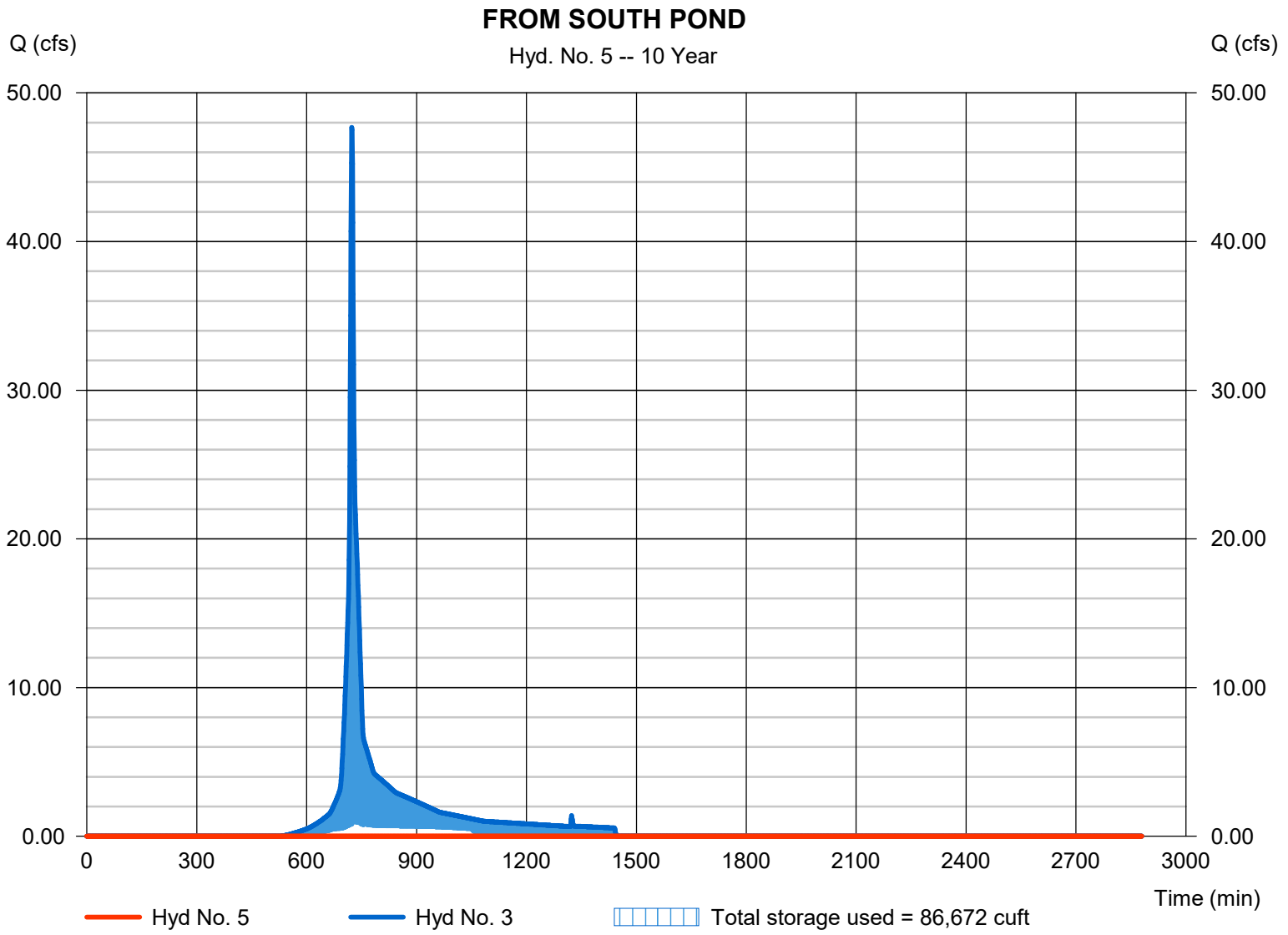
Hydrograph Report

Hyd. No. 5

FROM SOUTH POND

Hydrograph type	=	Reservoir	Peak discharge	=	0.000 cfs
Storm frequency	=	10 yrs	Time to peak	=	1587 min
Time interval	=	1 min	Hyd. volume	=	0 cuft
Inflow hyd. No.	=	3 - PRO DA-1	Max. Elevation	=	174.84 ft
Reservoir name	=	SOUTH POND	Max. Storage	=	86,672 cuft

Storage Indication method used. Exfiltration extracted from Outflow.



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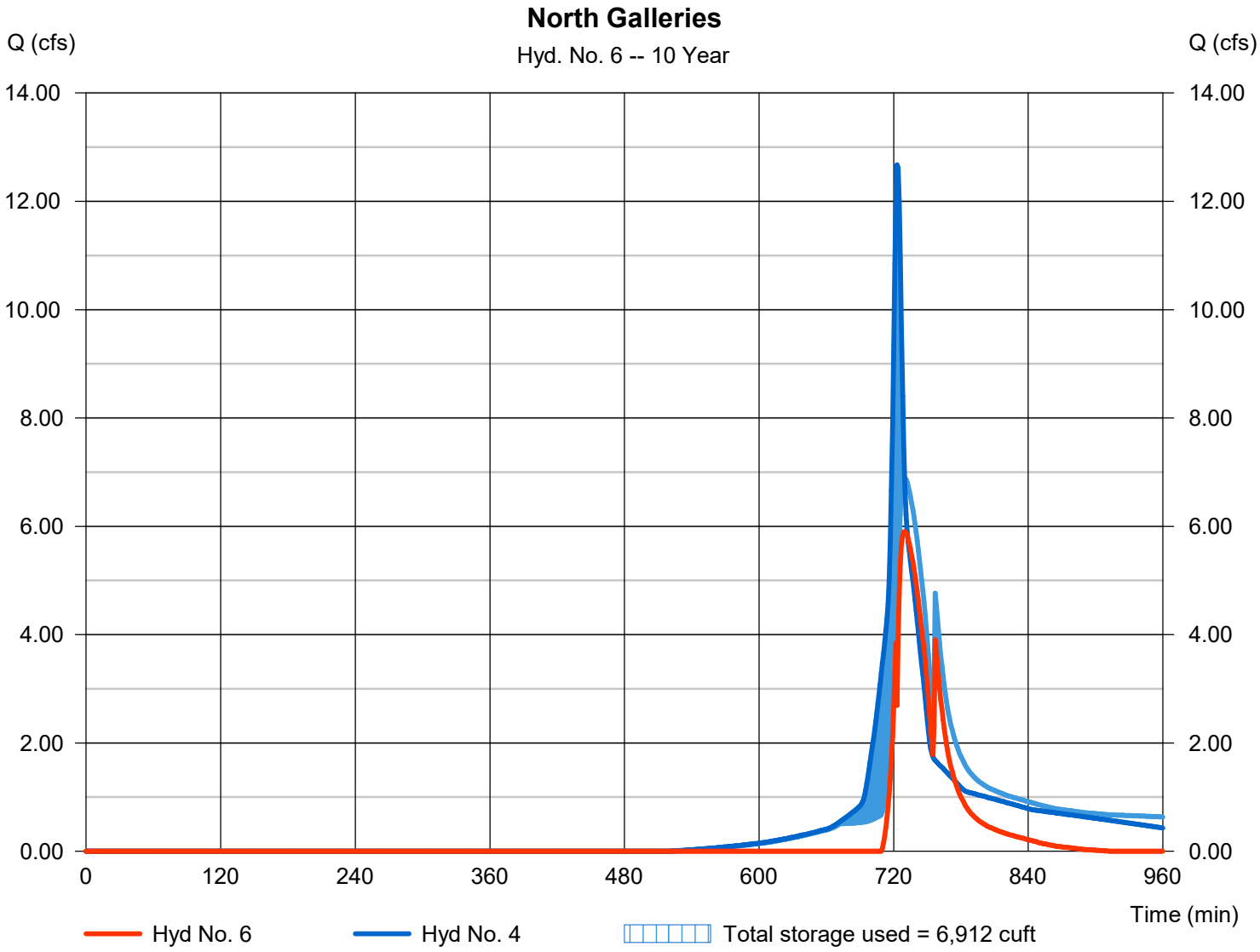
Hydrograph Report

Hyd. No. 6

North Galleries

Hydrograph type	=	Reservoir	Peak discharge	=	5.904 cfs
Storm frequency	=	10 yrs	Time to peak	=	730 min
Time interval	=	1 min	Hyd. volume	=	14,943 cuft
Inflow hyd. No.	=	4 - PROP DA-2	Max. Elevation	=	226.90 ft
Reservoir name	=	North Galleries	Max. Storage	=	6,912 cuft

Storage Indication method used. Exfiltration extracted from Outflow.



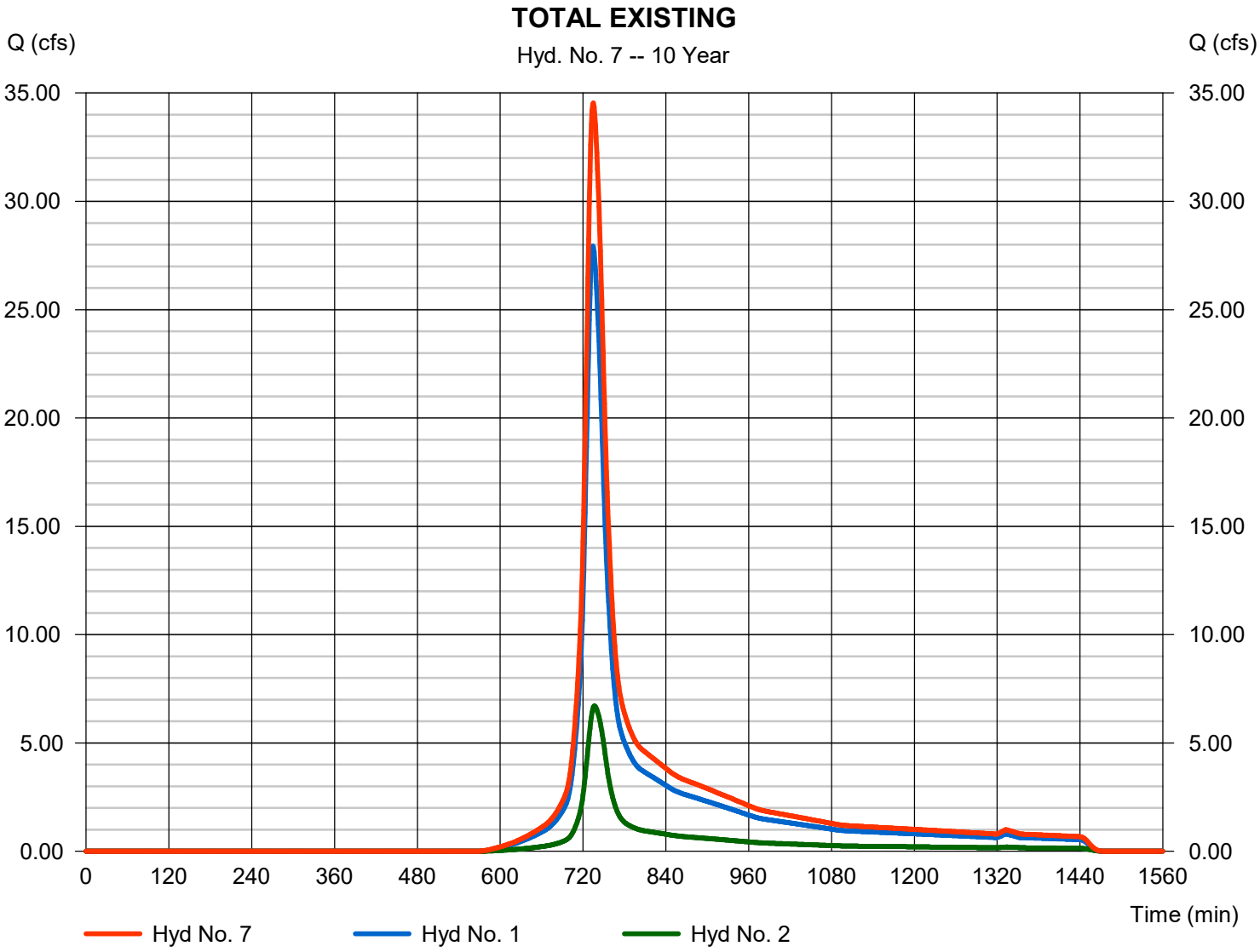
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Hydrograph Report

Hyd. No. 7

TOTAL EXISTING

Hydrograph type	=	Combine	Peak discharge	=	34.54 cfs
Storm frequency	=	10 yrs	Time to peak	=	735 min
Time interval	=	1 min	Hyd. volume	=	156,756 cuft
Inflow hyds.	=	1, 2	Contrib. drain. area	=	17.770 ac



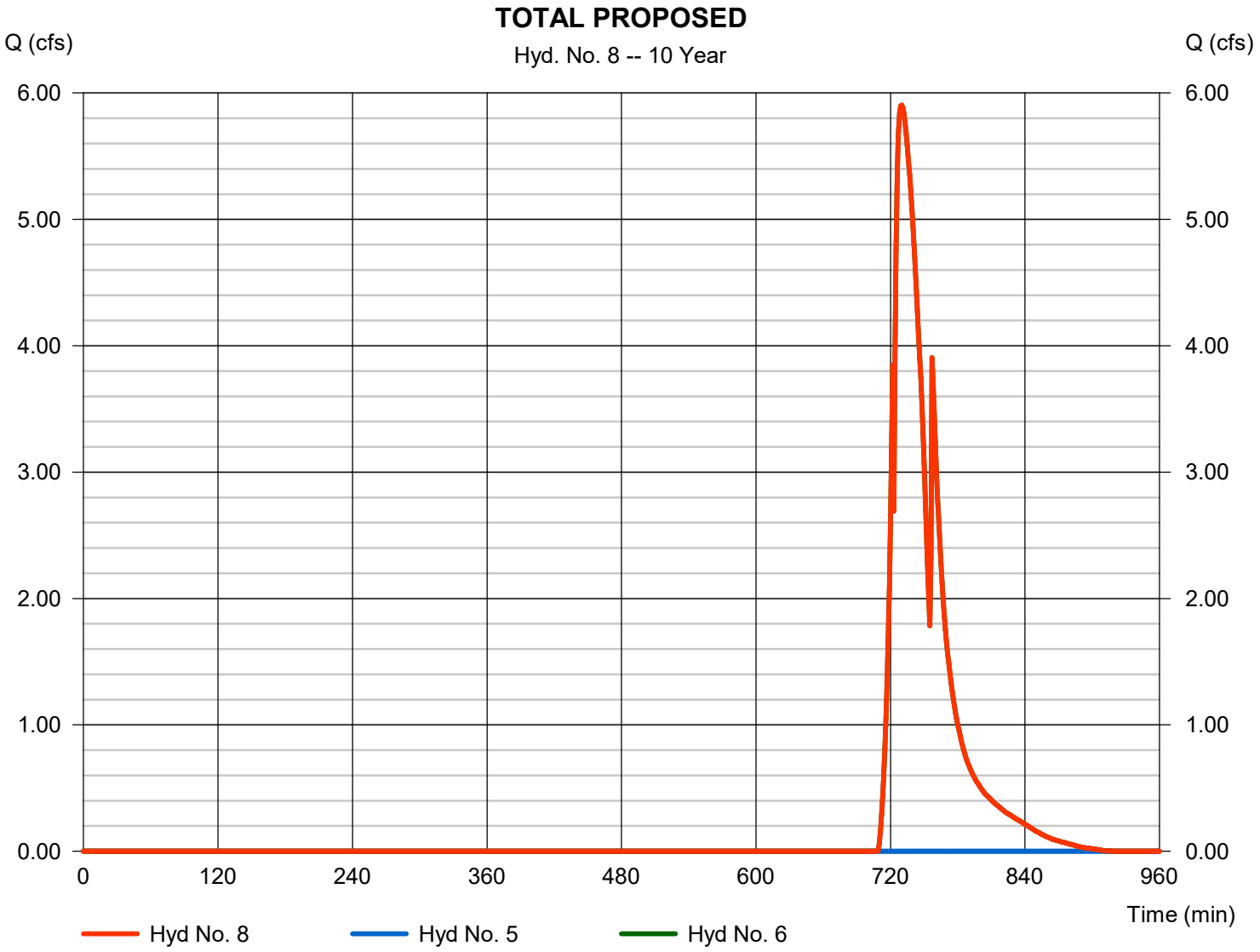
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Hydrograph Report

Hyd. No. 8

TOTAL PROPOSED

Hydrograph type	= Combine	Peak discharge	= 5.904 cfs
Storm frequency	= 10 yrs	Time to peak	= 730 min
Time interval	= 1 min	Hyd. volume	= 14,943 cuft
Inflow hyds.	= 5, 6	Contrib. drain. area	= 0.000 ac



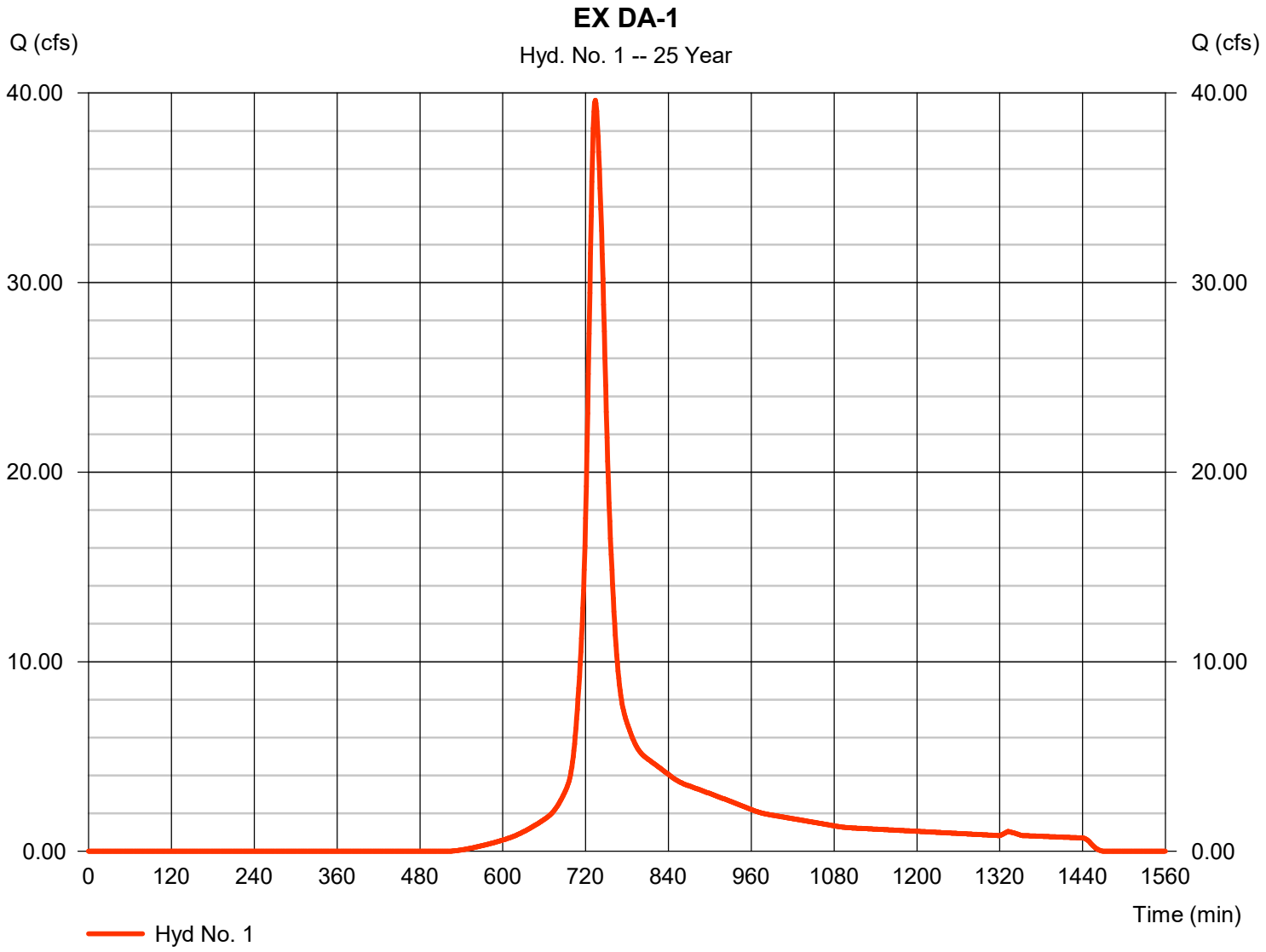
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Hydrograph Report

Hyd. No. 1

EX DA-1

Hydrograph type	=	SCS Runoff	Peak discharge	=	39.62 cfs
Storm frequency	=	25 yrs	Time to peak	=	734 min
Time interval	=	1 min	Hyd. volume	=	175,327 cuft
Drainage area	=	14.140 ac	Curve number	=	69
Basin Slope	=	0.0 %	Hydraulic length	=	0 ft
Tc method	=	TR55	Time of conc. (Tc)	=	18.80 min
Total precip.	=	6.88 in	Distribution	=	Type III
Storm duration	=	24 hrs	Shape factor	=	484



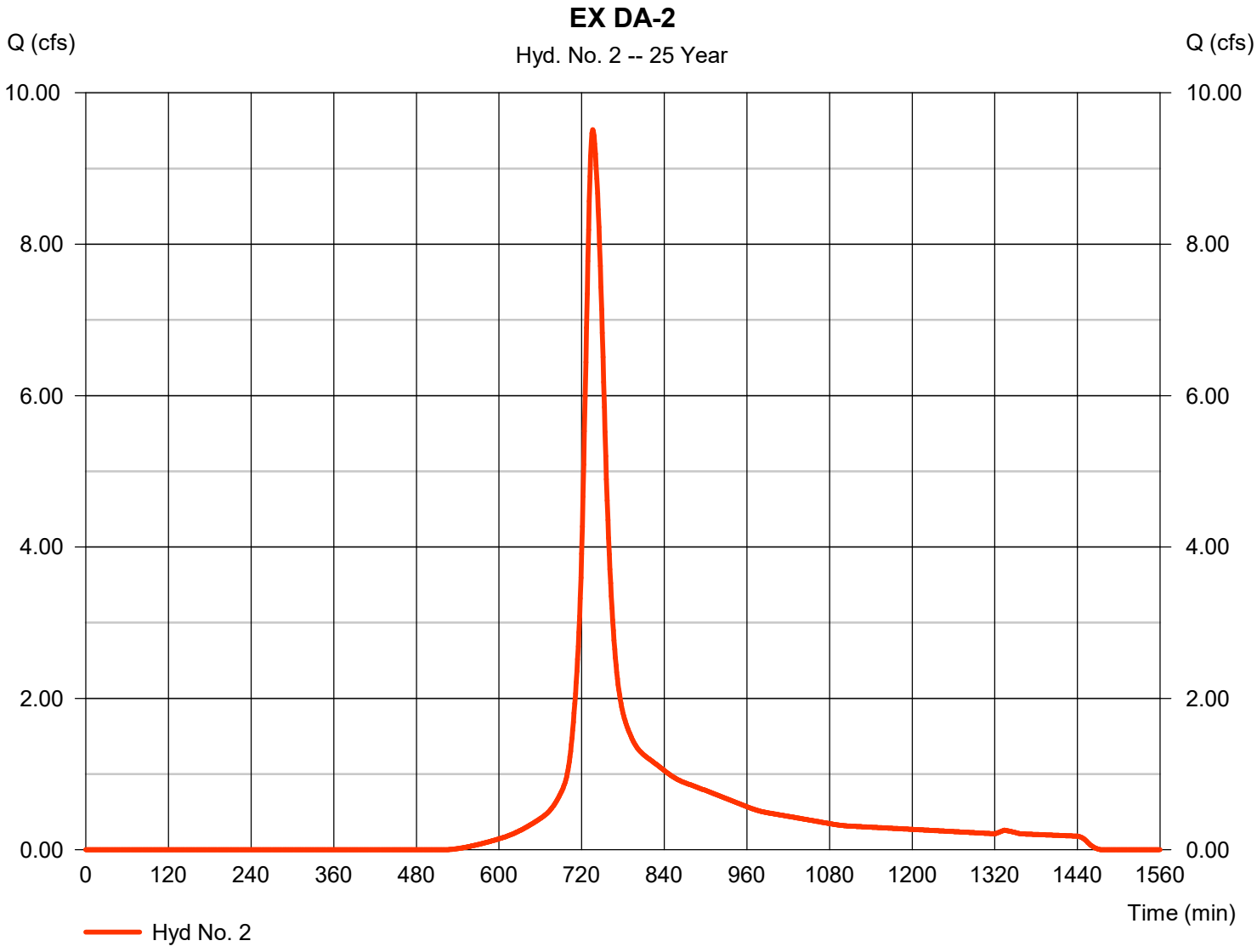
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Hydrograph Report

Hyd. No. 2

EX DA-2

Hydrograph type	=	SCS Runoff	Peak discharge	=	9.512 cfs
Storm frequency	=	25 yrs	Time to peak	=	736 min
Time interval	=	1 min	Hyd. volume	=	44,608 cuft
Drainage area	=	3.630 ac	Curve number	=	69
Basin Slope	=	0.0 %	Hydraulic length	=	0 ft
Tc method	=	TR55	Time of conc. (Tc)	=	22.70 min
Total precip.	=	6.88 in	Distribution	=	Type III
Storm duration	=	24 hrs	Shape factor	=	484



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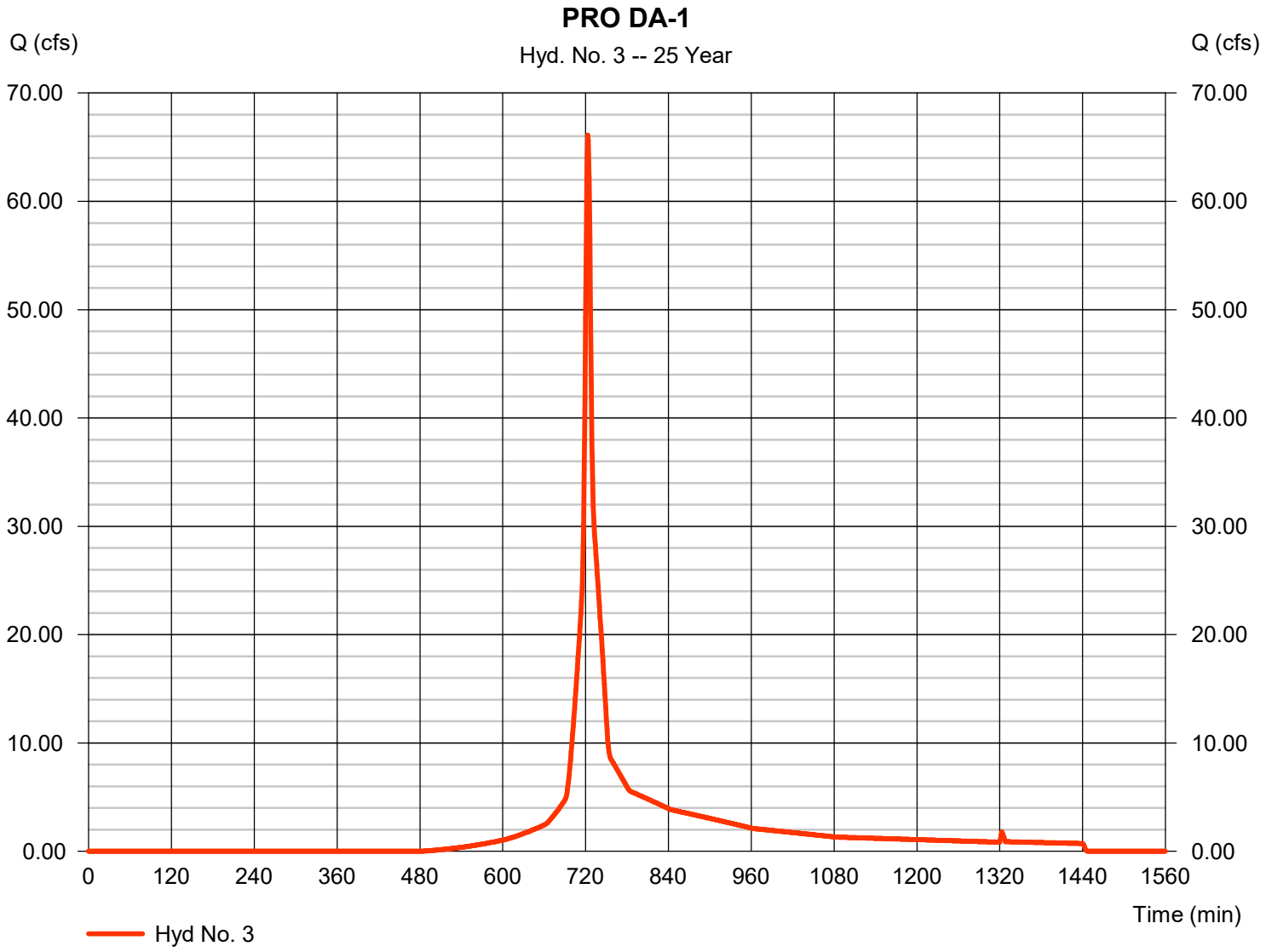
Hydrograph Report

Hyd. No. 3

PRO DA-1

Hydrograph type	=	SCS Runoff	Peak discharge	=	66.11 cfs
Storm frequency	=	25 yrs	Time to peak	=	723 min
Time interval	=	1 min	Hyd. volume	=	191,301 cuft
Drainage area	=	14.140 ac	Curve number	=	72*
Basin Slope	=	0.0 %	Hydraulic length	=	0 ft
Tc method	=	TR55	Time of conc. (Tc)	=	4.00 min
Total precip.	=	6.88 in	Distribution	=	Type III
Storm duration	=	24 hrs	Shape factor	=	484

* Composite (Area/CN) = [(1.660 x 98) + (12.480 x 69)] / 14.140



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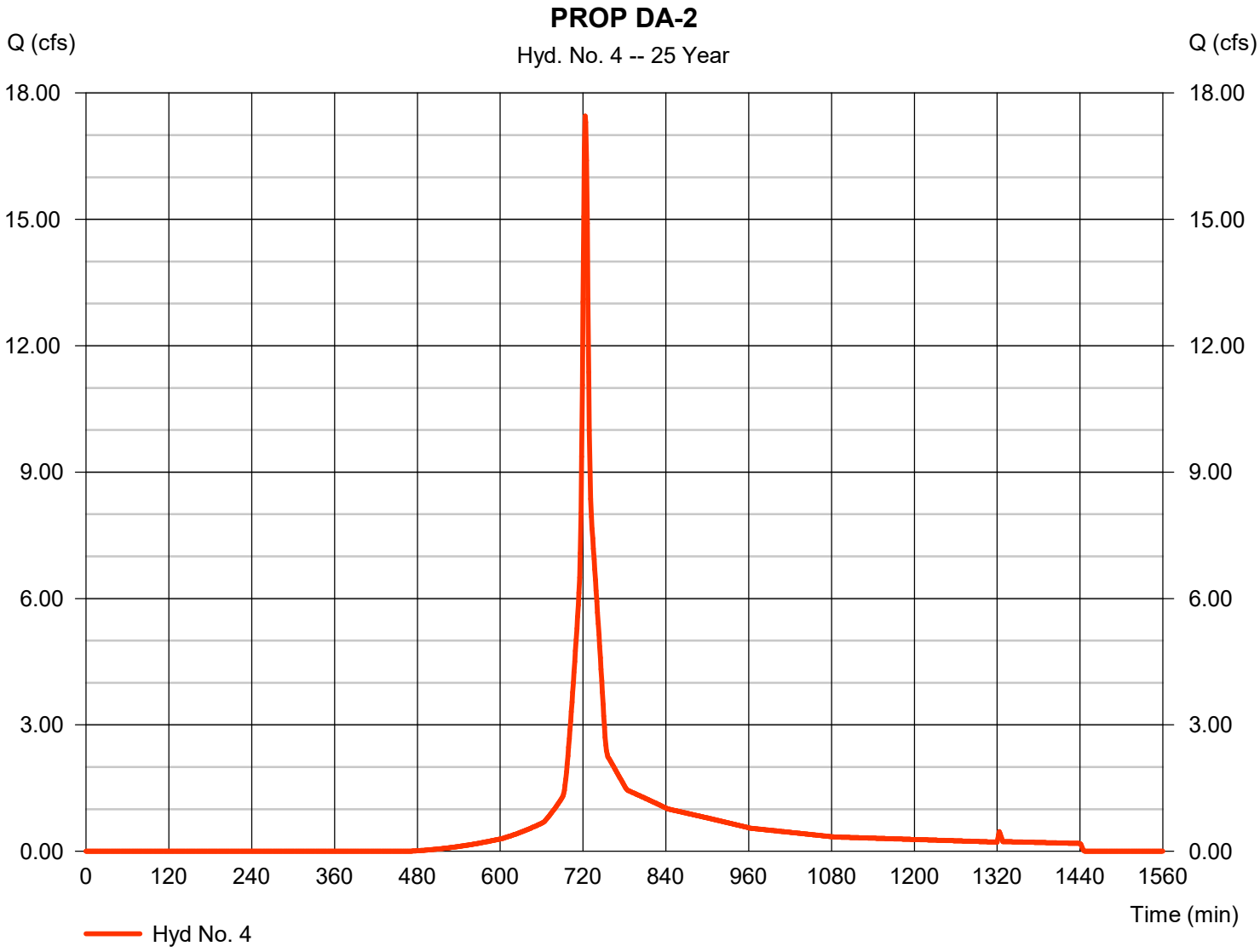
Hydrograph Report

Hyd. No. 4

PROP DA-2

Hydrograph type	=	SCS Runoff	Peak discharge	=	17.45 cfs
Storm frequency	=	25 yrs	Time to peak	=	723 min
Time interval	=	1 min	Hyd. volume	=	50,494 cuft
Drainage area	=	3.630 ac	Curve number	=	73*
Basin Slope	=	0.0 %	Hydraulic length	=	0 ft
Tc method	=	TR55	Time of conc. (Tc)	=	3.90 min
Total precip.	=	6.88 in	Distribution	=	Type III
Storm duration	=	24 hrs	Shape factor	=	484

* Composite (Area/CN) = [(0.471 x 98) + (3.160 x 69)] / 3.630



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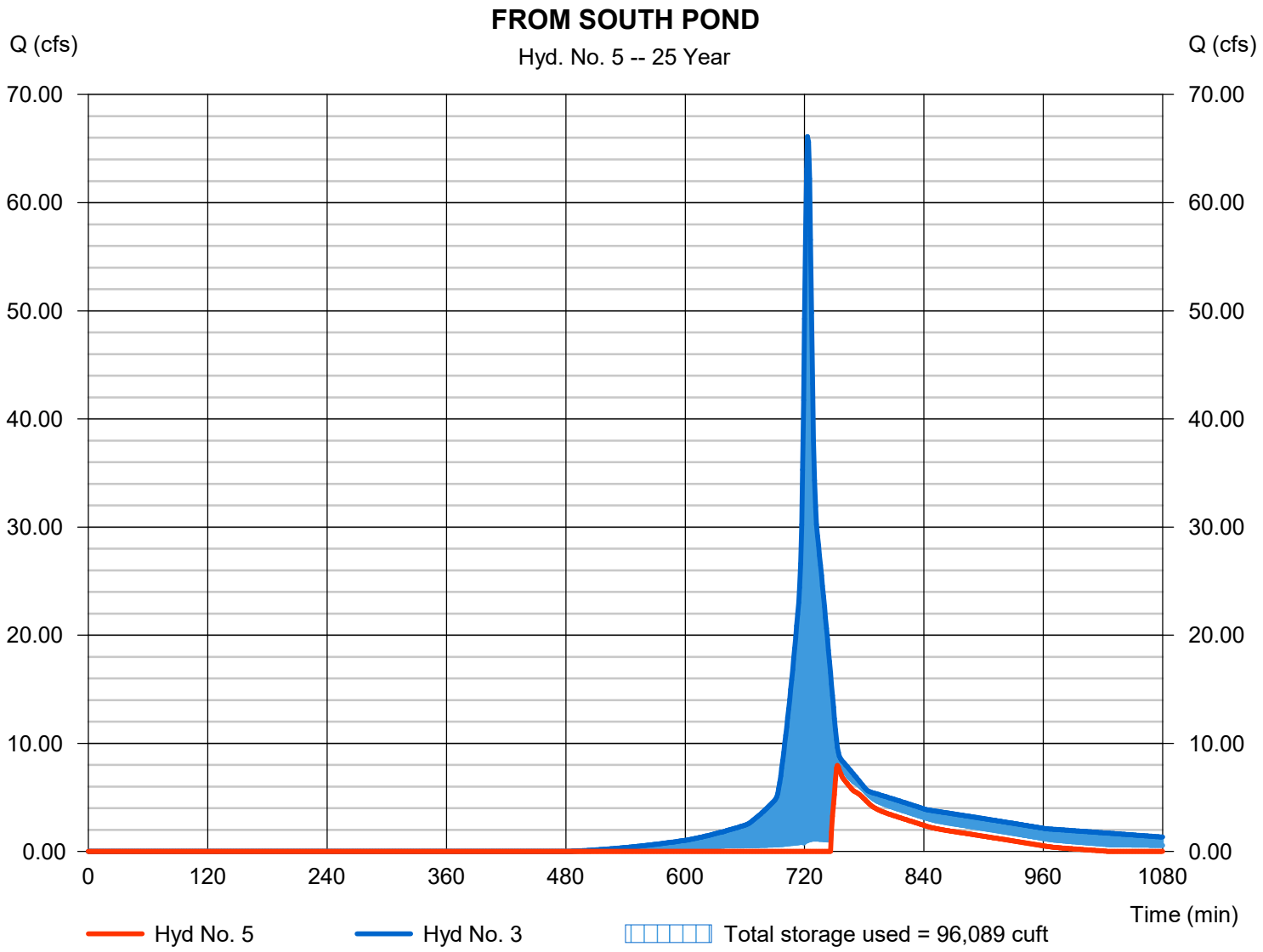
Hydrograph Report

Hyd. No. 5

FROM SOUTH POND

Hydrograph type	=	Reservoir	Peak discharge	=	7.942 cfs
Storm frequency	=	25 yrs	Time to peak	=	753 min
Time interval	=	1 min	Hyd. volume	=	34,991 cuft
Inflow hyd. No.	=	3 - PRO DA-1	Max. Elevation	=	175.62 ft
Reservoir name	=	SOUTH POND	Max. Storage	=	96,089 cuft

Storage Indication method used. Exfiltration extracted from Outflow.



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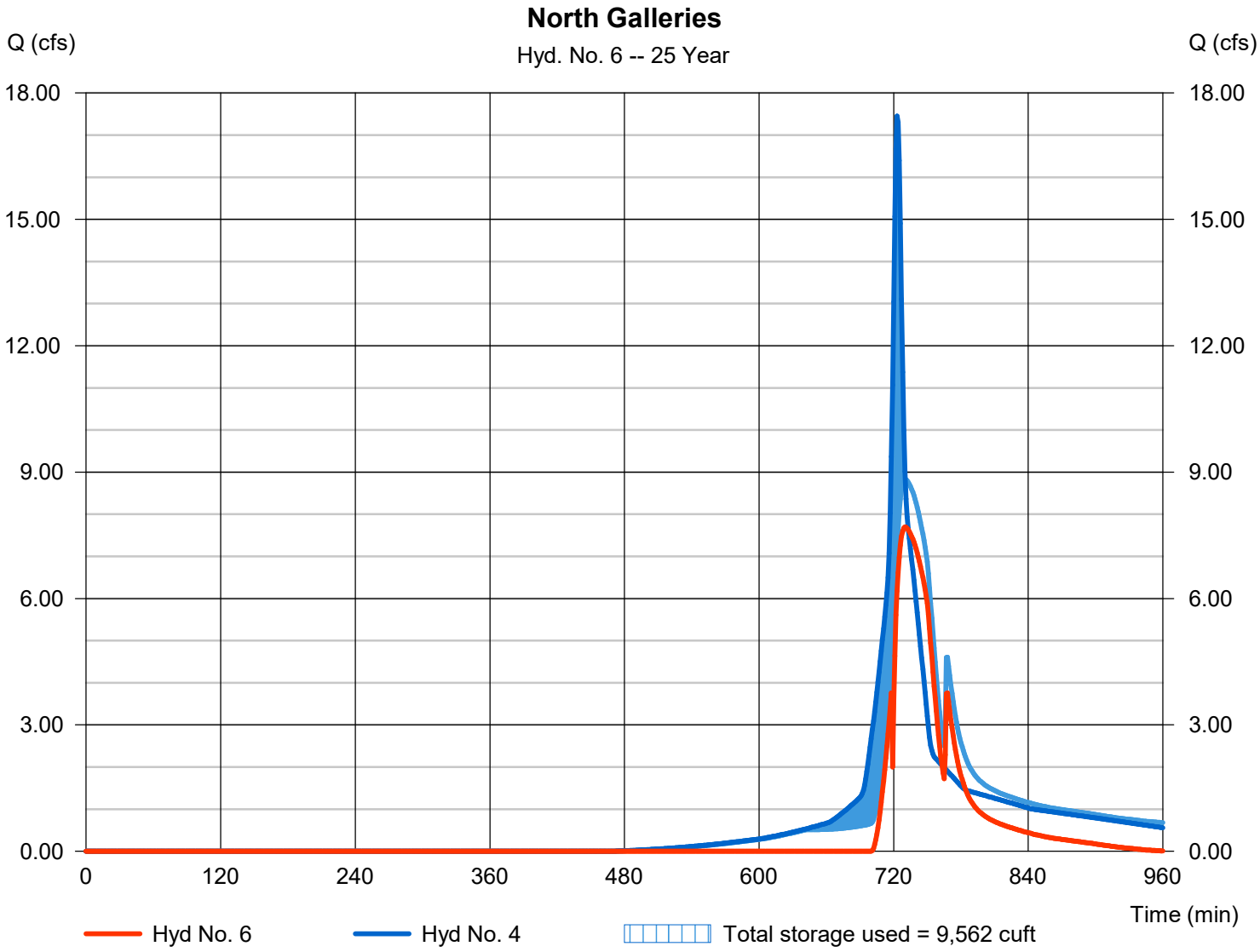
Hydrograph Report

Hyd. No. 6

North Galleries

Hydrograph type	=	Reservoir	Peak discharge	=	7.696 cfs
Storm frequency	=	25 yrs	Time to peak	=	730 min
Time interval	=	1 min	Hyd. volume	=	24,165 cuft
Inflow hyd. No.	=	4 - PROP DA-2	Max. Elevation	=	227.82 ft
Reservoir name	=	North Galleries	Max. Storage	=	9,562 cuft

Storage Indication method used. Exfiltration extracted from Outflow.



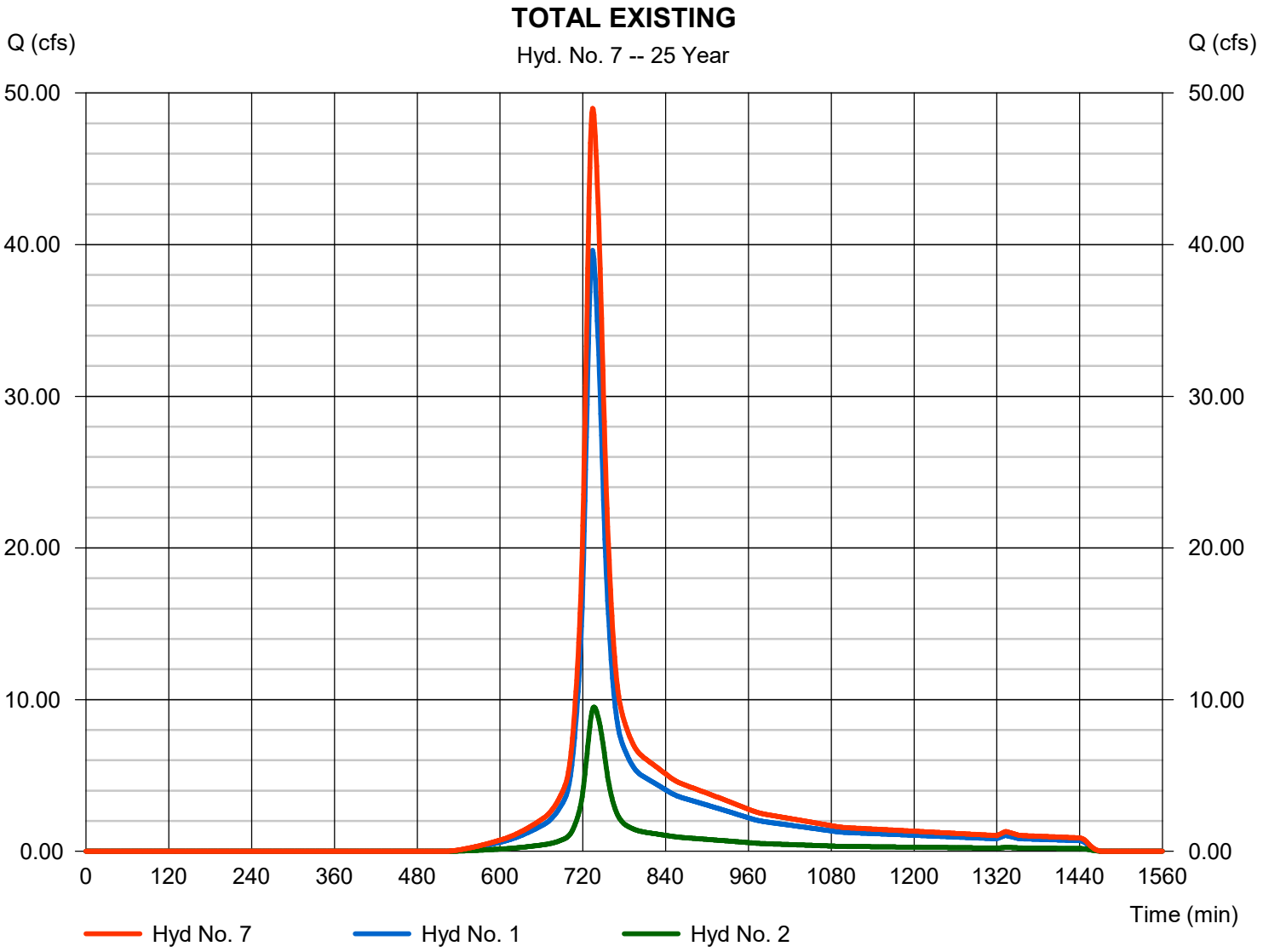
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Hydrograph Report

Hyd. No. 7

TOTAL EXISTING

Hydrograph type	=	Combine	Peak discharge	=	48.98 cfs
Storm frequency	=	25 yrs	Time to peak	=	734 min
Time interval	=	1 min	Hyd. volume	=	219,935 cuft
Inflow hyds.	=	1, 2	Contrib. drain. area	=	17.770 ac



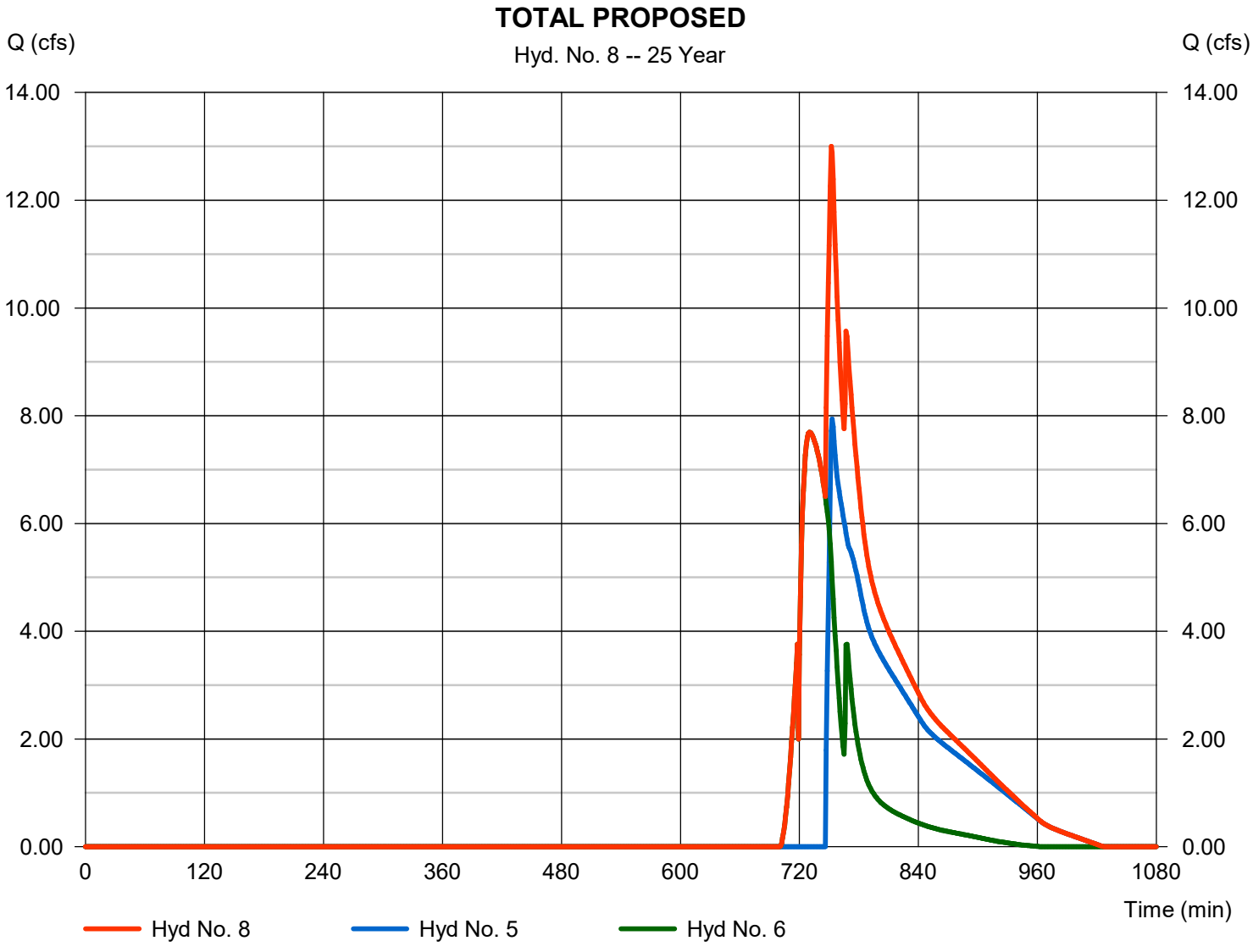
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Hydrograph Report

Hyd. No. 8

TOTAL PROPOSED

Hydrograph type	=	Combine	Peak discharge	=	13.00 cfs
Storm frequency	=	25 yrs	Time to peak	=	752 min
Time interval	=	1 min	Hyd. volume	=	59,156 cuft
Inflow hyds.	=	5, 6	Contrib. drain. area	=	0.000 ac



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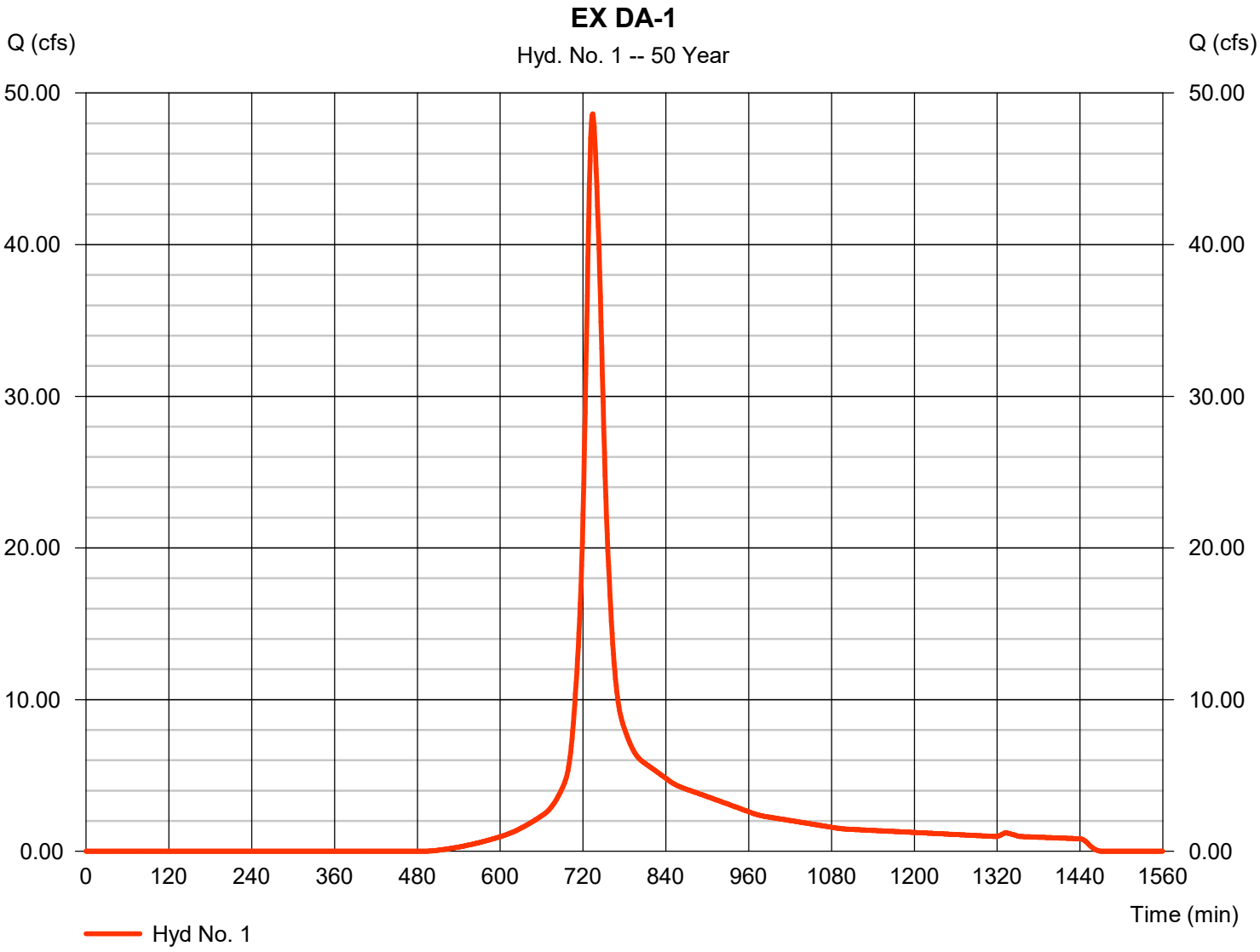
Hydrograph Report

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2022 Tuesday, 03 / 3 / 2026

Hyd. No. 1

EX DA-1

Hydrograph type	=	SCS Runoff	Peak discharge	=	48.62 cfs
Storm frequency	=	50 yrs	Time to peak	=	734 min
Time interval	=	1 min	Hyd. volume	=	214,562 cuft
Drainage area	=	14.140 ac	Curve number	=	69
Basin Slope	=	0.0 %	Hydraulic length	=	0 ft
Tc method	=	TR55	Time of conc. (Tc)	=	18.80 min
Total precip.	=	7.80 in	Distribution	=	Type III
Storm duration	=	24 hrs	Shape factor	=	484



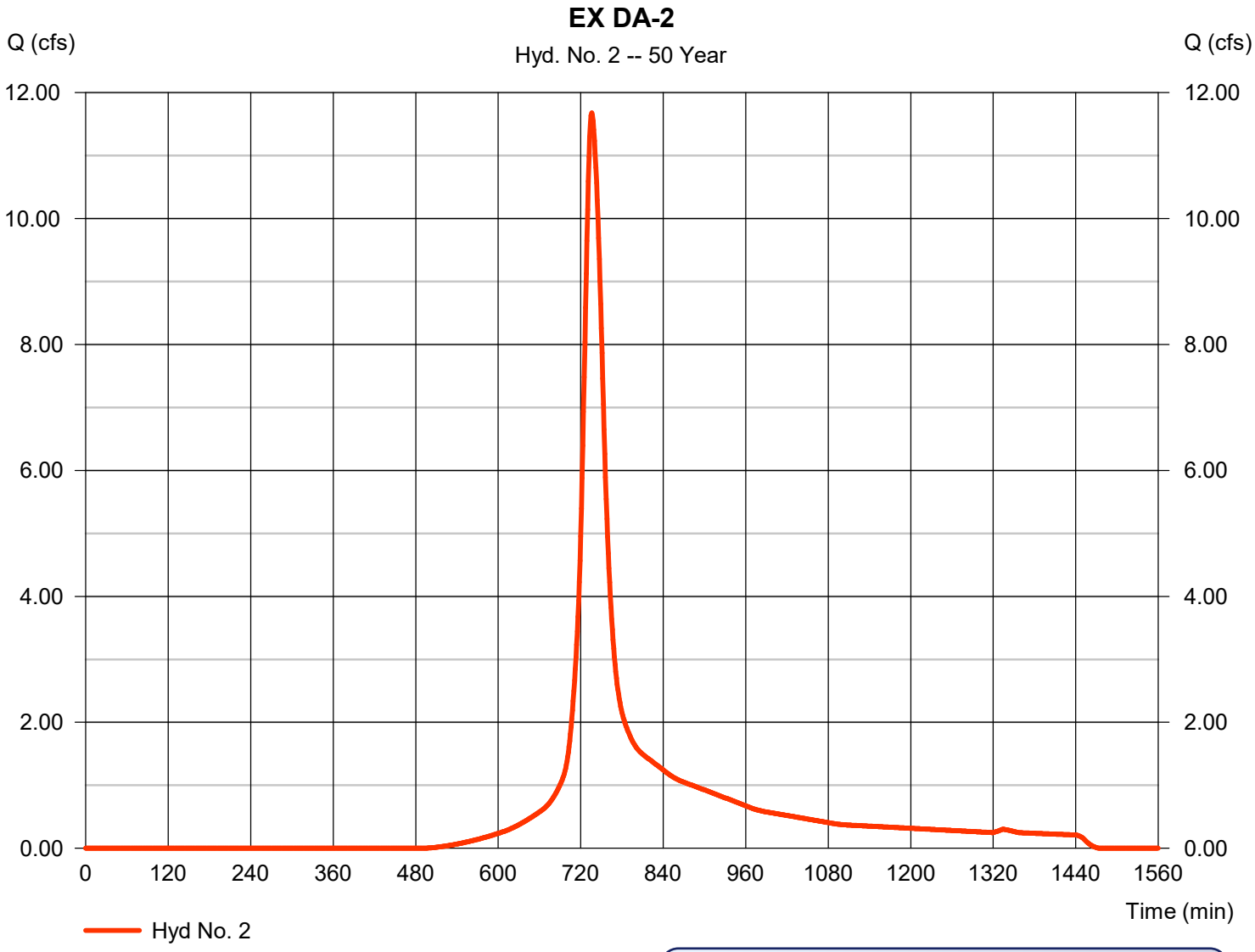
RECEIVED
By Marc J. Garofalo, MPA, MCC, MCTC at 10:44 am, Apr 14, 2026

Hydrograph Report

Hyd. No. 2

EX DA-2

Hydrograph type	=	SCS Runoff	Peak discharge	=	11.68 cfs
Storm frequency	=	50 yrs	Time to peak	=	736 min
Time interval	=	1 min	Hyd. volume	=	54,590 cuft
Drainage area	=	3.630 ac	Curve number	=	69
Basin Slope	=	0.0 %	Hydraulic length	=	0 ft
Tc method	=	TR55	Time of conc. (Tc)	=	22.70 min
Total precip.	=	7.80 in	Distribution	=	Type III
Storm duration	=	24 hrs	Shape factor	=	484



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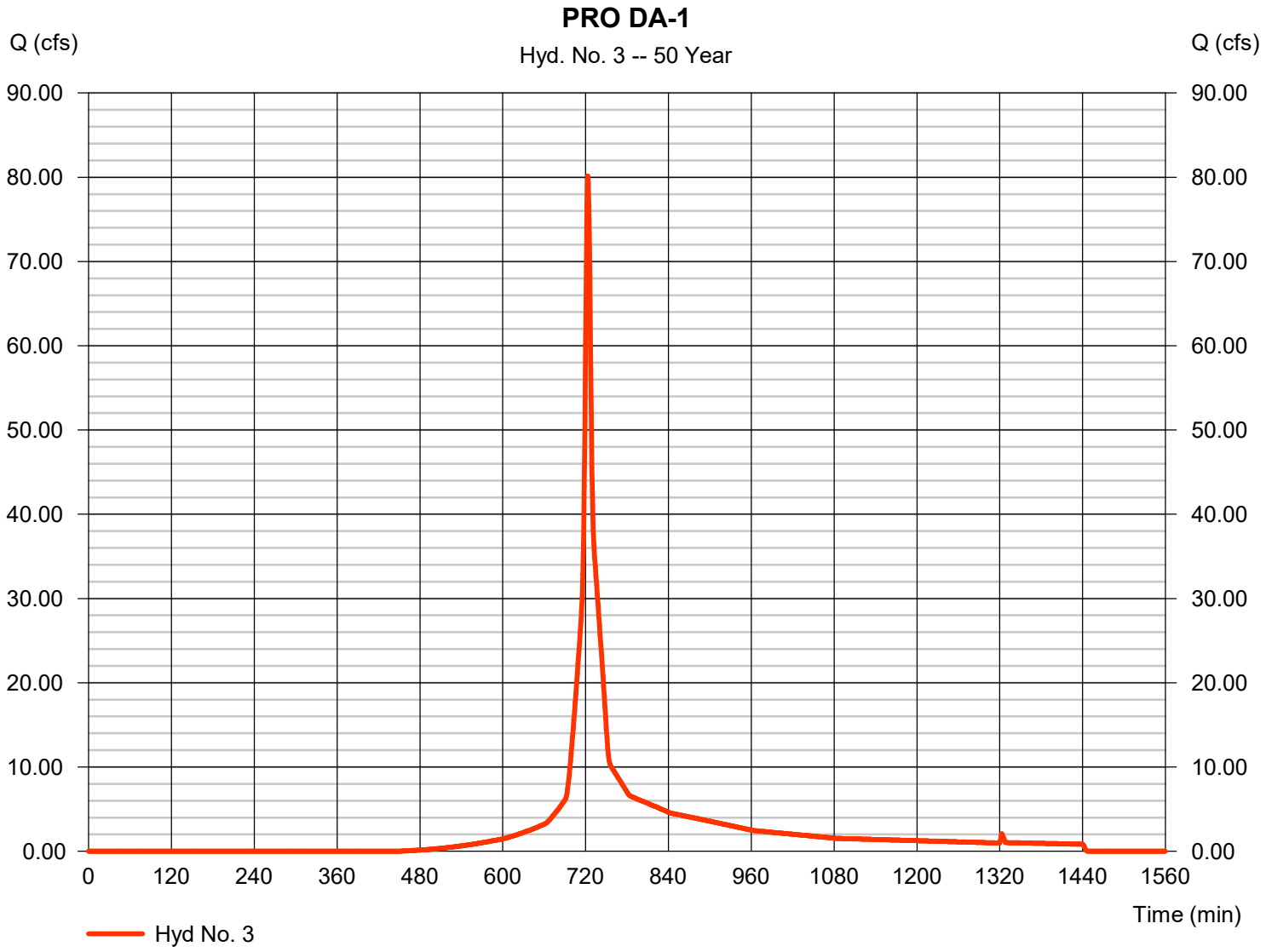
Hydrograph Report

Hyd. No. 3

PRO DA-1

Hydrograph type	=	SCS Runoff	Peak discharge	=	80.16 cfs
Storm frequency	=	50 yrs	Time to peak	=	723 min
Time interval	=	1 min	Hyd. volume	=	231,972 cuft
Drainage area	=	14.140 ac	Curve number	=	72*
Basin Slope	=	0.0 %	Hydraulic length	=	0 ft
Tc method	=	TR55	Time of conc. (Tc)	=	4.00 min
Total precip.	=	7.80 in	Distribution	=	Type III
Storm duration	=	24 hrs	Shape factor	=	484

* Composite (Area/CN) = [(1.660 x 98) + (12.480 x 69)] / 14.140



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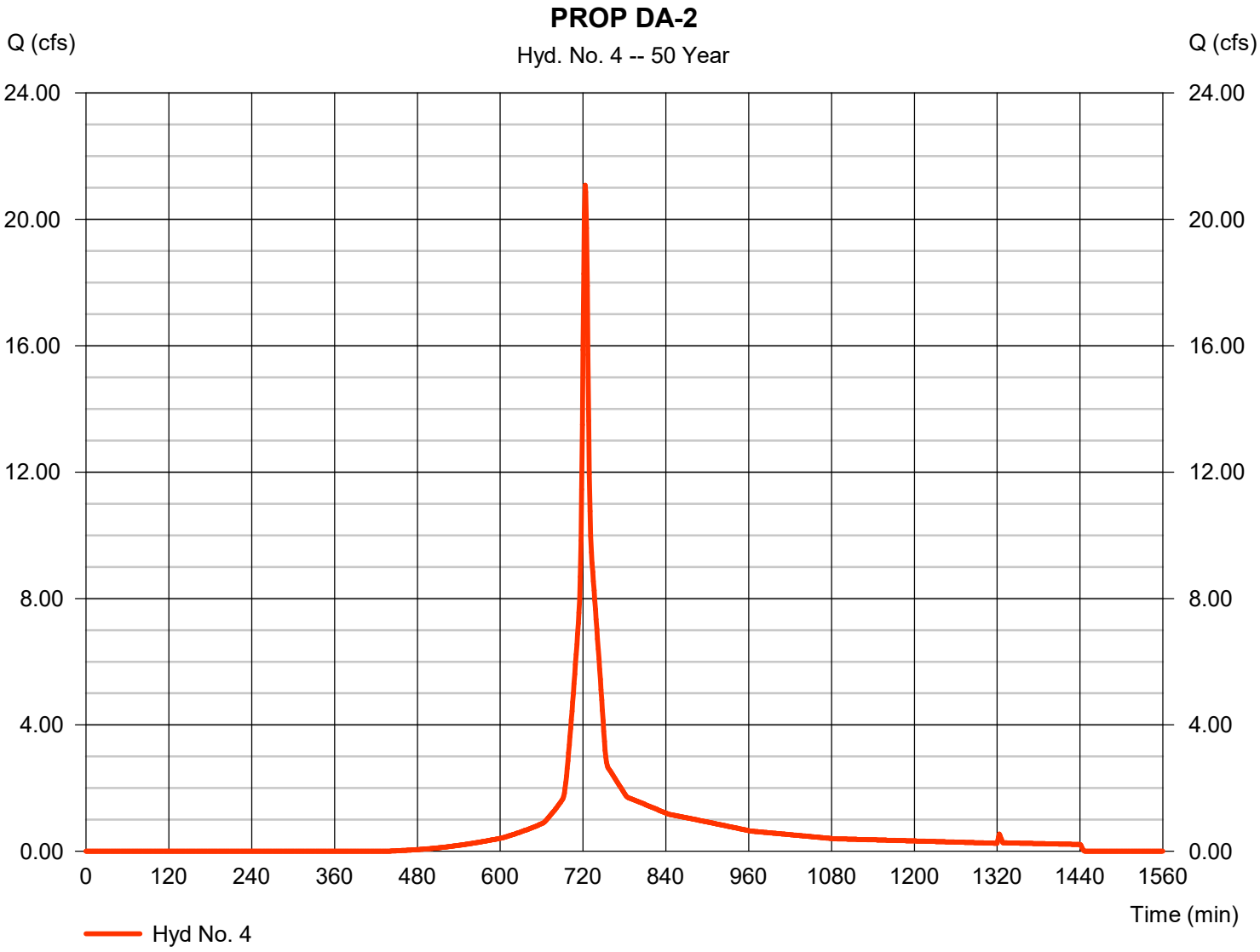
Hydrograph Report

Hyd. No. 4

PROP DA-2

Hydrograph type	=	SCS Runoff	Peak discharge	=	21.08 cfs
Storm frequency	=	50 yrs	Time to peak	=	723 min
Time interval	=	1 min	Hyd. volume	=	61,050 cuft
Drainage area	=	3.630 ac	Curve number	=	73*
Basin Slope	=	0.0 %	Hydraulic length	=	0 ft
Tc method	=	TR55	Time of conc. (Tc)	=	3.90 min
Total precip.	=	7.80 in	Distribution	=	Type III
Storm duration	=	24 hrs	Shape factor	=	484

* Composite (Area/CN) = [(0.471 x 98) + (3.160 x 69)] / 3.630



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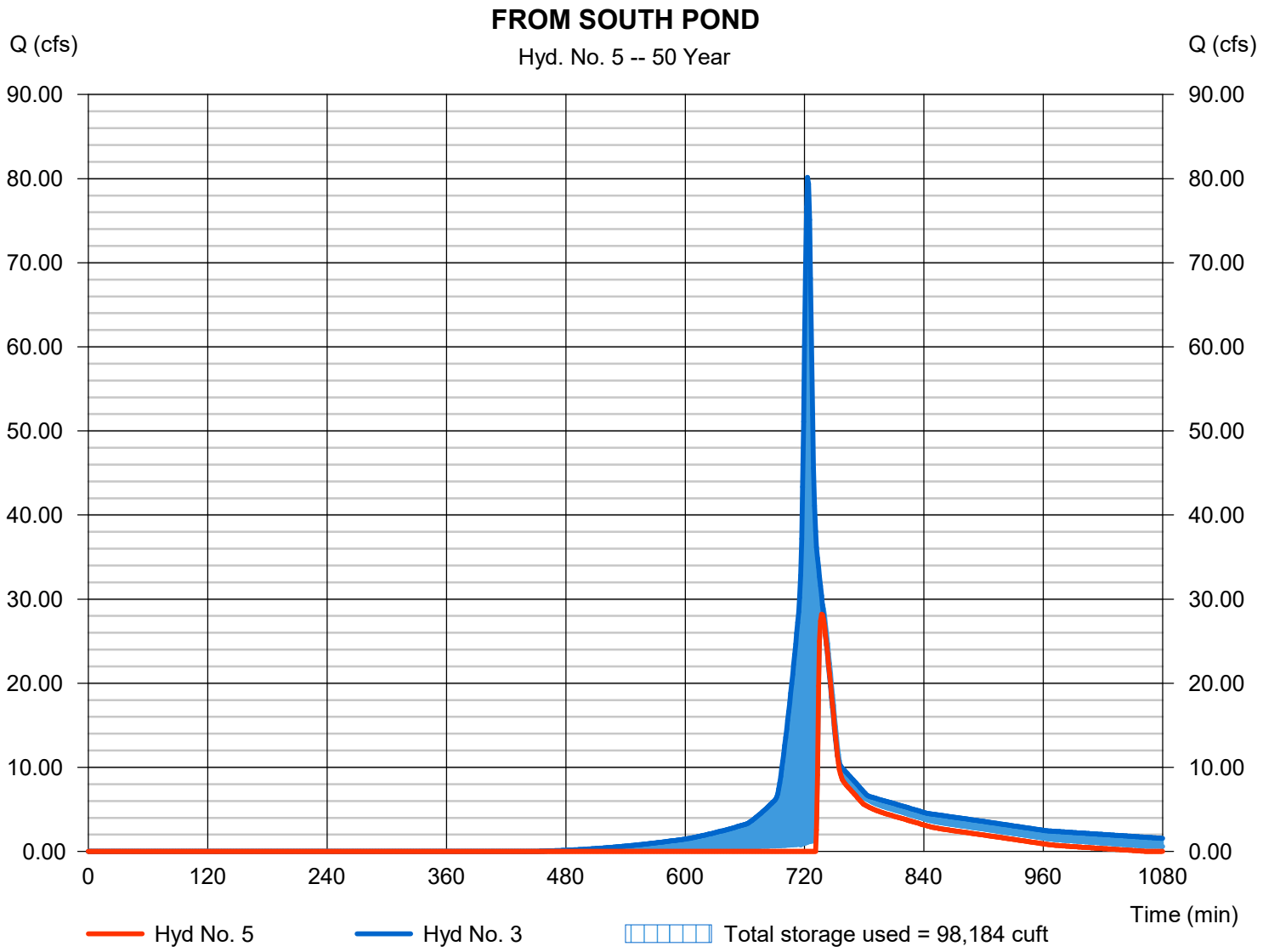
Hydrograph Report

Hyd. No. 5

FROM SOUTH POND

Hydrograph type	=	Reservoir	Peak discharge	=	28.21 cfs
Storm frequency	=	50 yrs	Time to peak	=	737 min
Time interval	=	1 min	Hyd. volume	=	69,849 cuft
Inflow hyd. No.	=	3 - PRO DA-1	Max. Elevation	=	175.79 ft
Reservoir name	=	SOUTH POND	Max. Storage	=	98,184 cuft

Storage Indication method used. Exfiltration extracted from Outflow.



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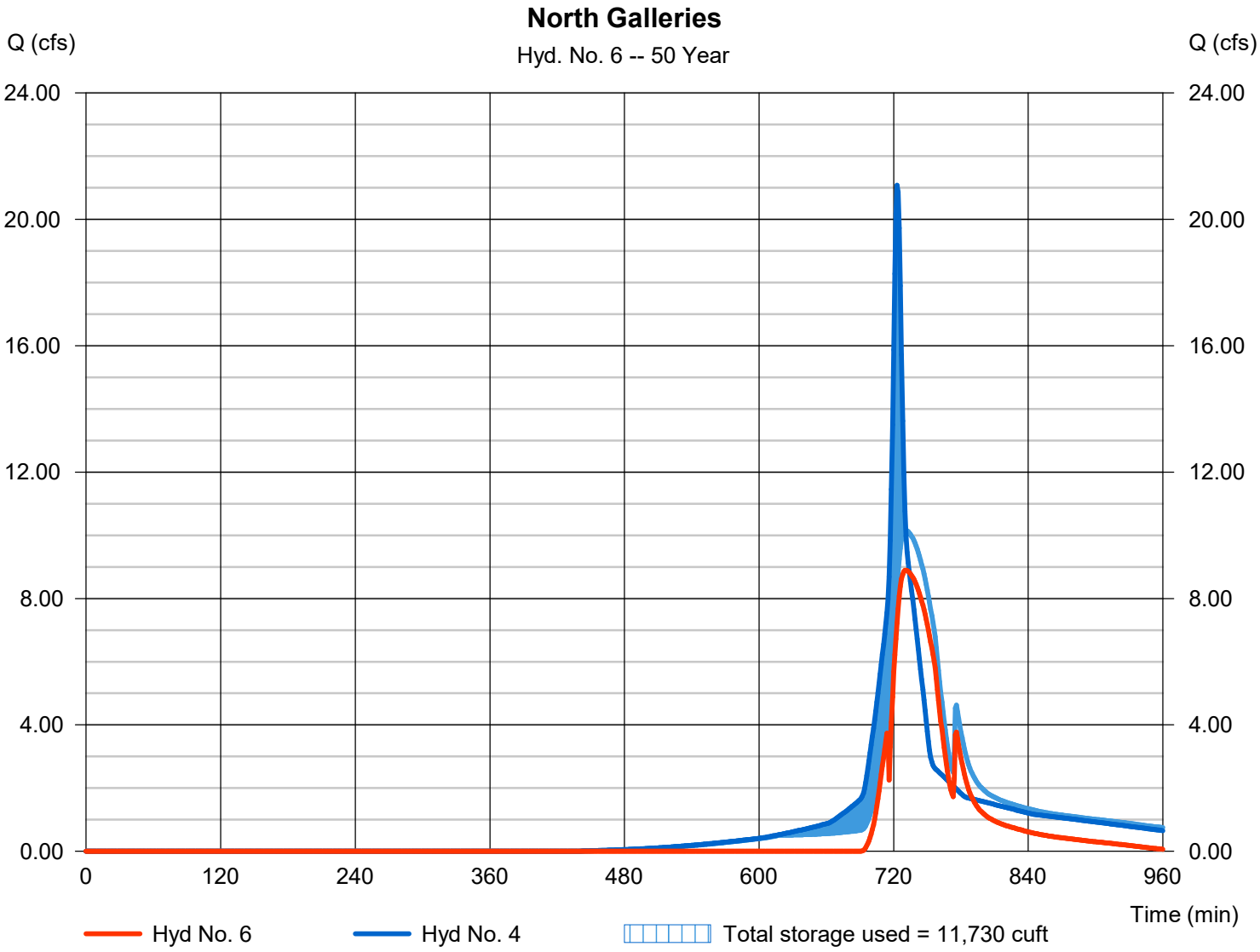
Hydrograph Report

Hyd. No. 6

North Galleries

Hydrograph type	=	Reservoir	Peak discharge	=	8.898 cfs
Storm frequency	=	50 yrs	Time to peak	=	731 min
Time interval	=	1 min	Hyd. volume	=	31,633 cuft
Inflow hyd. No.	=	4 - PROP DA-2	Max. Elevation	=	228.57 ft
Reservoir name	=	North Galleries	Max. Storage	=	11,730 cuft

Storage Indication method used. Exfiltration extracted from Outflow.



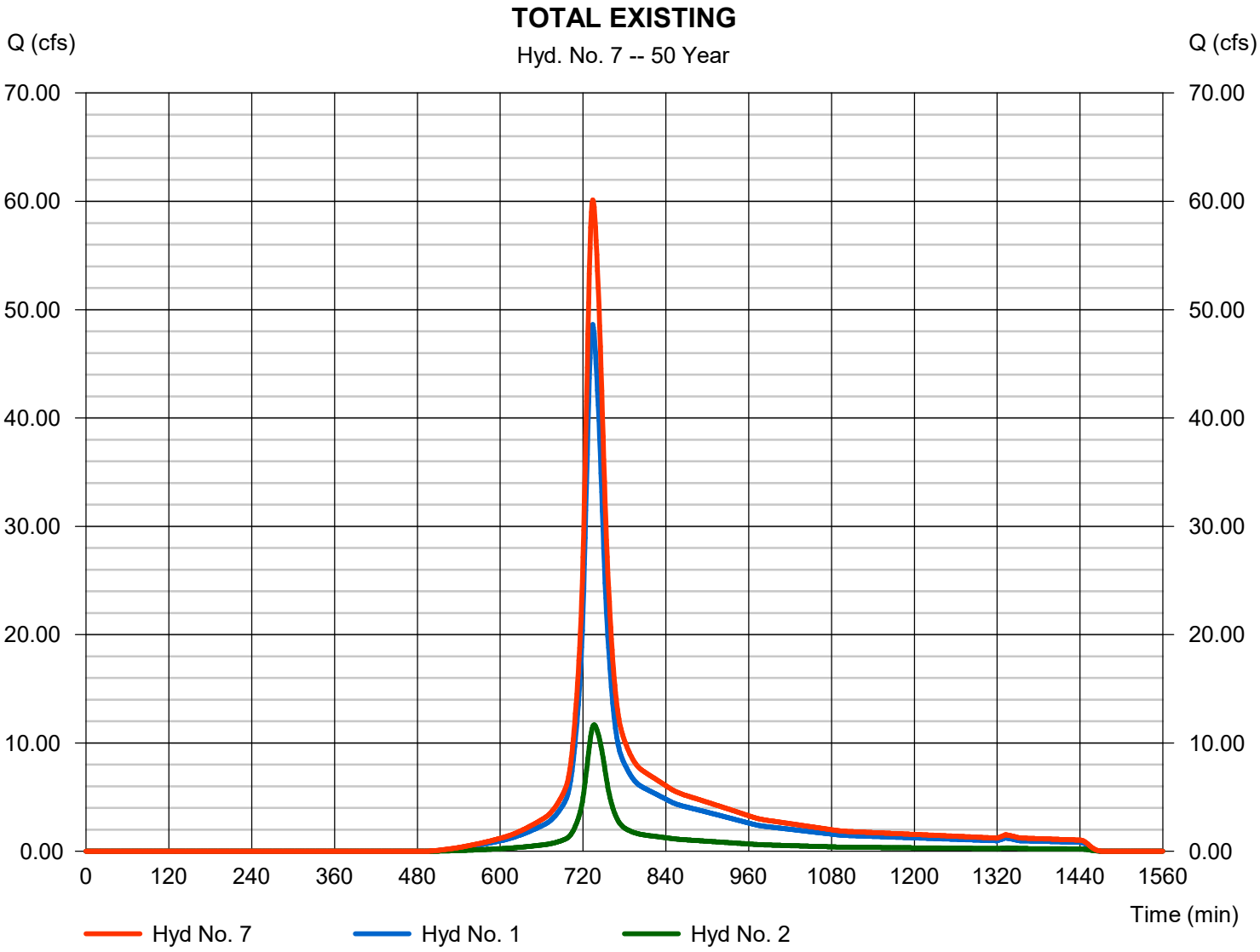
RECEIVED
By Marc J. Garofalo, MPA, MCC, MCTC at 10:44 am, Apr 14, 2026

Hydrograph Report

Hyd. No. 7

TOTAL EXISTING

Hydrograph type	=	Combine	Peak discharge	=	60.13 cfs
Storm frequency	=	50 yrs	Time to peak	=	734 min
Time interval	=	1 min	Hyd. volume	=	269,152 cuft
Inflow hyds.	=	1, 2	Contrib. drain. area	=	17.770 ac



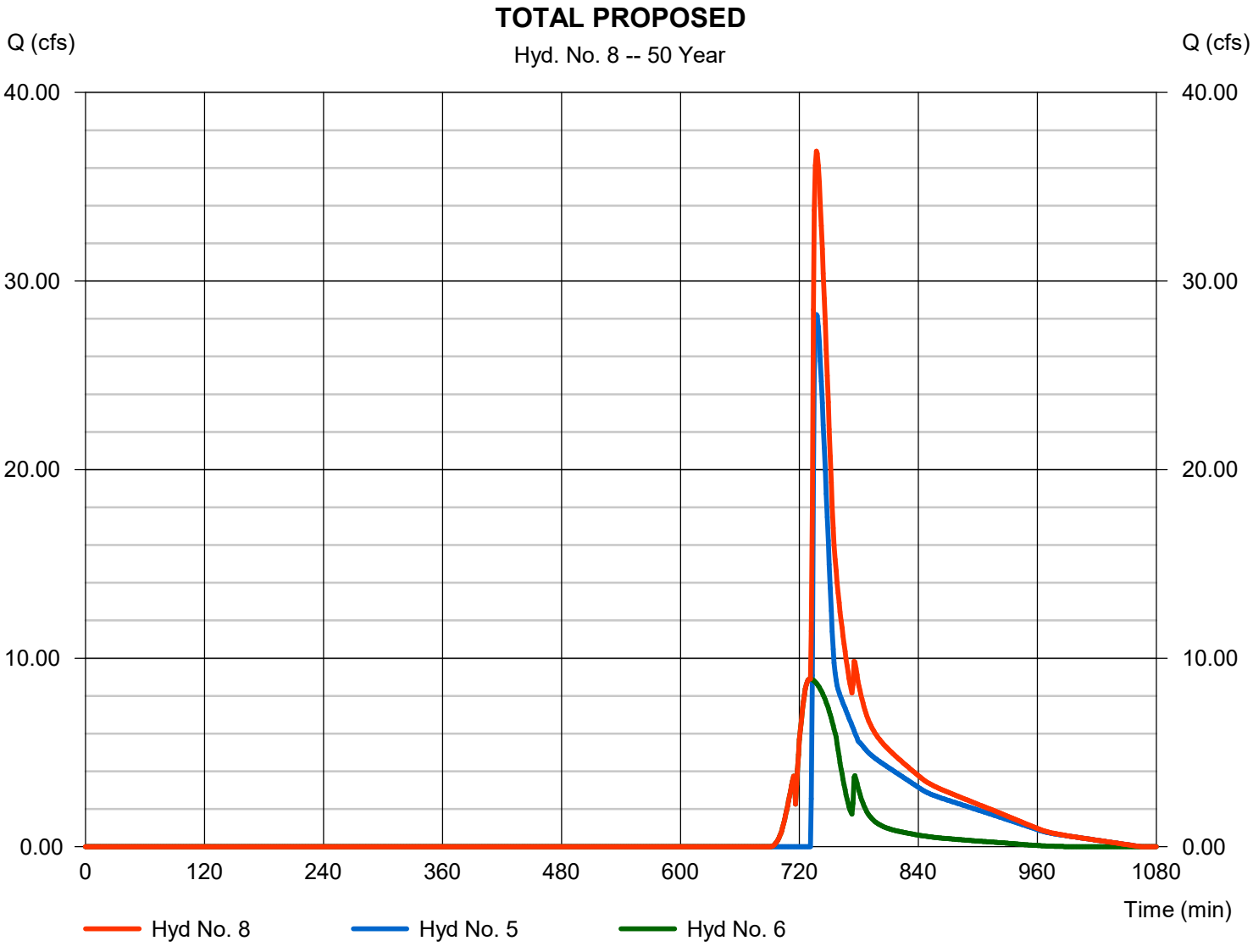
RECEIVED
By Marc J. Garofalo, MPA, MCC, MCTC at 10:44 am, Apr 14, 2026

Hydrograph Report

Hyd. No. 8

TOTAL PROPOSED

Hydrograph type	=	Combine	Peak discharge	=	36.89 cfs
Storm frequency	=	50 yrs	Time to peak	=	737 min
Time interval	=	1 min	Hyd. volume	=	101,481 cuft
Inflow hyds.	=	5, 6	Contrib. drain. area	=	0.000 ac



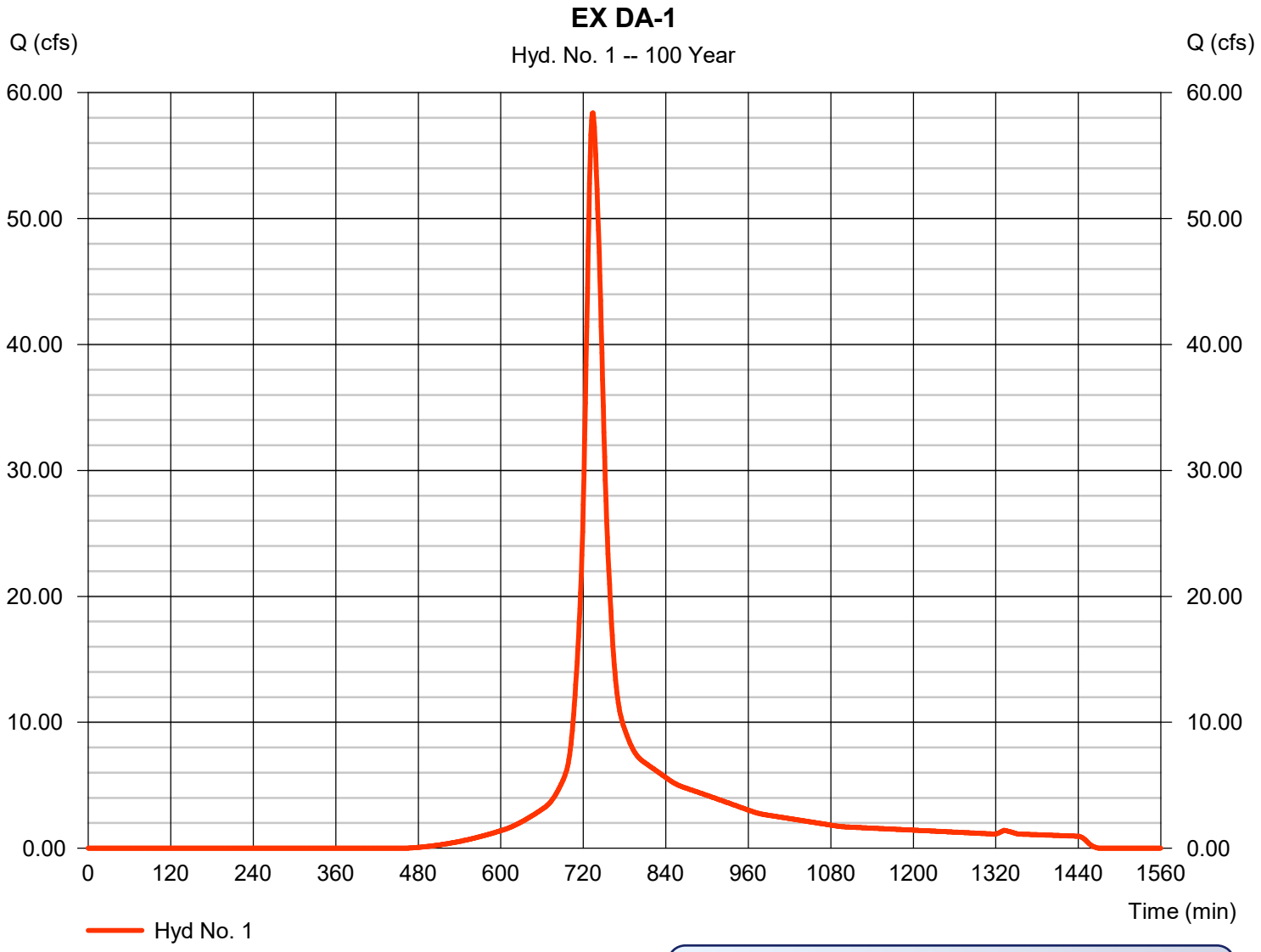
RECEIVED
By Marc J. Garofalo, MPA, MCC, MCTC at 10:44 am, Apr 14, 2026

Hydrograph Report

Hyd. No. 1

EX DA-1

Hydrograph type	=	SCS Runoff	Peak discharge	=	58.40 cfs
Storm frequency	=	100 yrs	Time to peak	=	734 min
Time interval	=	1 min	Hyd. volume	=	257,662 cuft
Drainage area	=	14.140 ac	Curve number	=	69
Basin Slope	=	0.0 %	Hydraulic length	=	0 ft
Tc method	=	TR55	Time of conc. (Tc)	=	18.80 min
Total precip.	=	8.78 in	Distribution	=	Type III
Storm duration	=	24 hrs	Shape factor	=	484



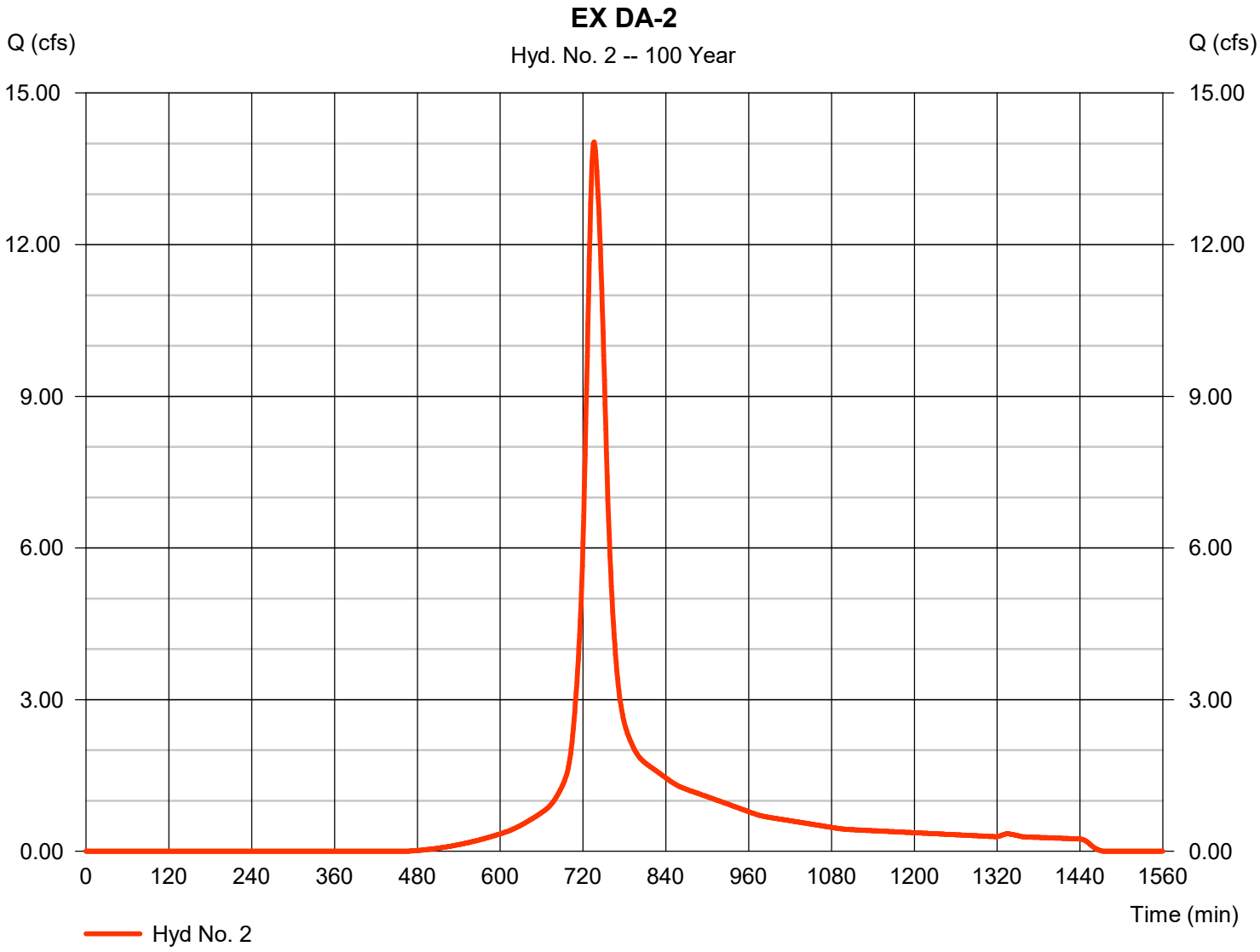
RECEIVED
By Marc J. Garofalo, MPA, MCC, MCTC at 10:45 am, Apr 14, 2026

Hydrograph Report

Hyd. No. 2

EX DA-2

Hydrograph type	=	SCS Runoff	Peak discharge	=	14.03 cfs
Storm frequency	=	100 yrs	Time to peak	=	736 min
Time interval	=	1 min	Hyd. volume	=	65,556 cuft
Drainage area	=	3.630 ac	Curve number	=	69
Basin Slope	=	0.0 %	Hydraulic length	=	0 ft
Tc method	=	TR55	Time of conc. (Tc)	=	22.70 min
Total precip.	=	8.78 in	Distribution	=	Type III
Storm duration	=	24 hrs	Shape factor	=	484



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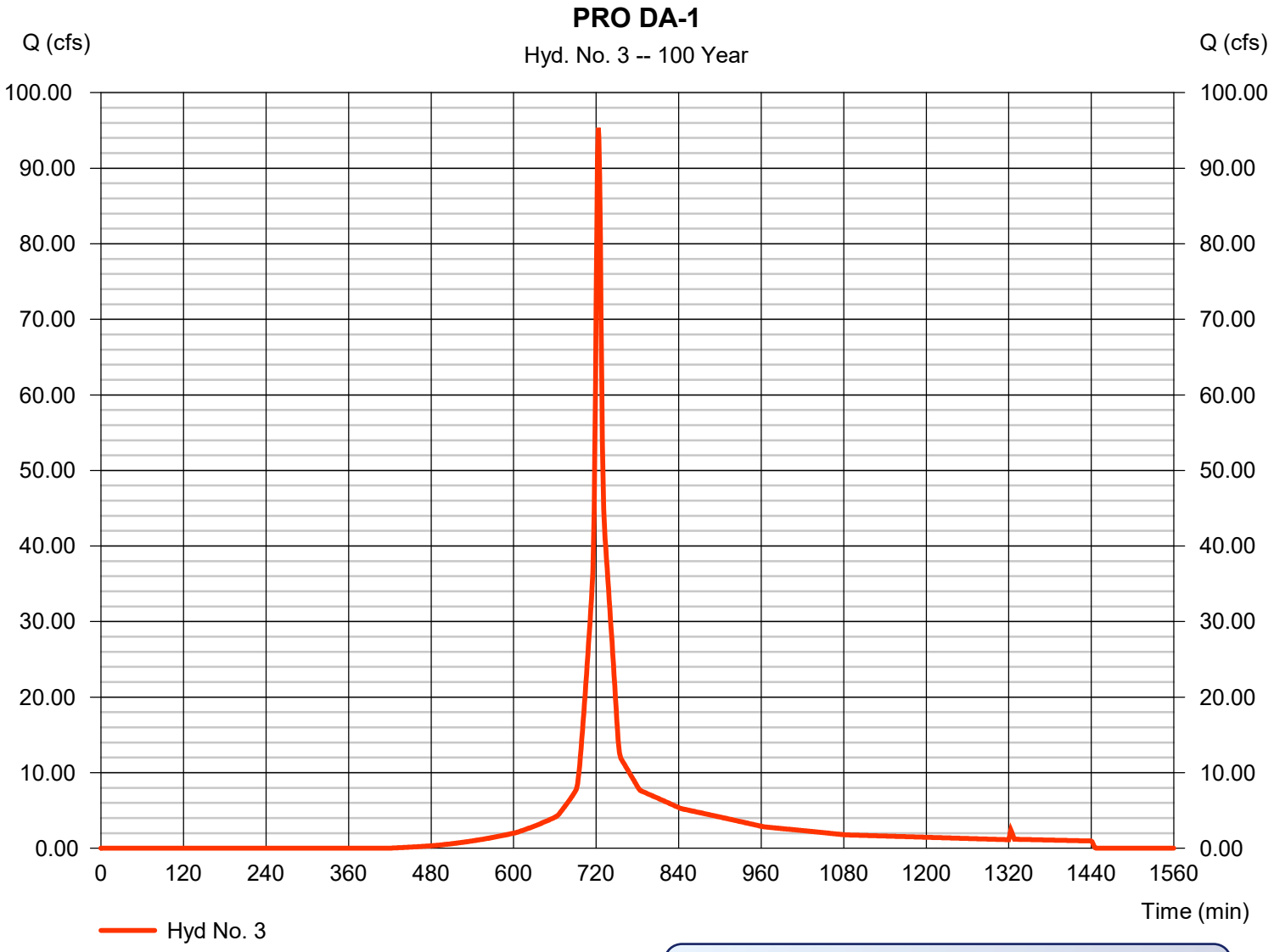
Hydrograph Report

Hyd. No. 3

PRO DA-1

Hydrograph type	=	SCS Runoff	Peak discharge	=	95.33 cfs
Storm frequency	=	100 yrs	Time to peak	=	723 min
Time interval	=	1 min	Hyd. volume	=	276,411 cuft
Drainage area	=	14.140 ac	Curve number	=	72*
Basin Slope	=	0.0 %	Hydraulic length	=	0 ft
Tc method	=	TR55	Time of conc. (Tc)	=	4.00 min
Total precip.	=	8.78 in	Distribution	=	Type III
Storm duration	=	24 hrs	Shape factor	=	484

* Composite (Area/CN) = [(1.660 x 98) + (12.480 x 69)] / 14.140



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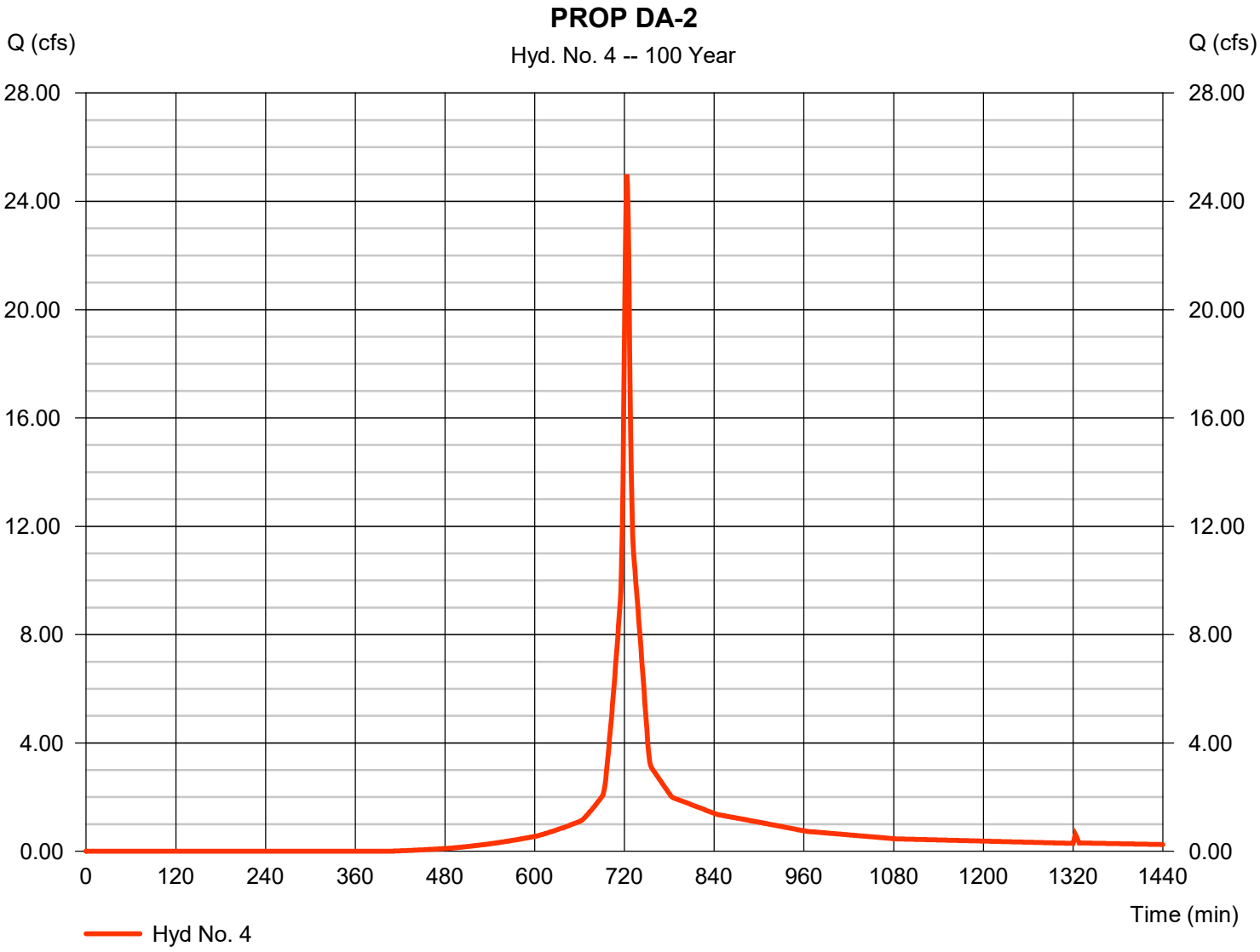
Hydrograph Report

Hyd. No. 4

PROP DA-2

Hydrograph type	=	SCS Runoff	Peak discharge	=	24.99 cfs
Storm frequency	=	100 yrs	Time to peak	=	723 min
Time interval	=	1 min	Hyd. volume	=	72,565 cuft
Drainage area	=	3.630 ac	Curve number	=	73*
Basin Slope	=	0.0 %	Hydraulic length	=	0 ft
Tc method	=	TR55	Time of conc. (Tc)	=	3.90 min
Total precip.	=	8.78 in	Distribution	=	Type III
Storm duration	=	24 hrs	Shape factor	=	484

* Composite (Area/CN) = [(0.471 x 98) + (3.160 x 69)] / 3.630



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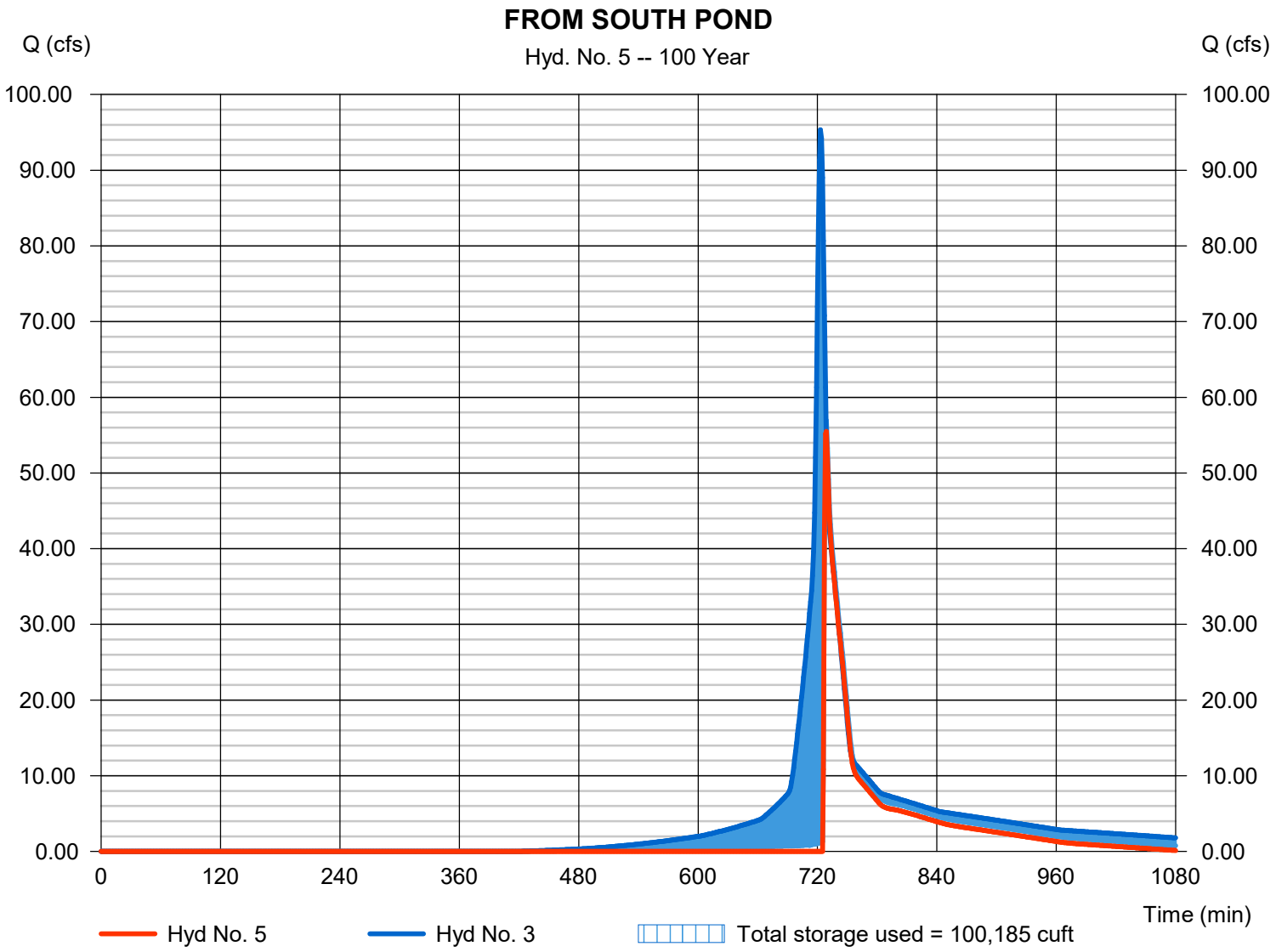
Hydrograph Report

Hyd. No. 5

FROM SOUTH POND

Hydrograph type	=	Reservoir	Peak discharge	=	55.50 cfs
Storm frequency	=	100 yrs	Time to peak	=	729 min
Time interval	=	1 min	Hyd. volume	=	108,741 cuft
Inflow hyd. No.	=	3 - PRO DA-1	Max. Elevation	=	175.96 ft
Reservoir name	=	SOUTH POND	Max. Storage	=	100,185 cuft

Storage Indication method used. Exfiltration extracted from Outflow.



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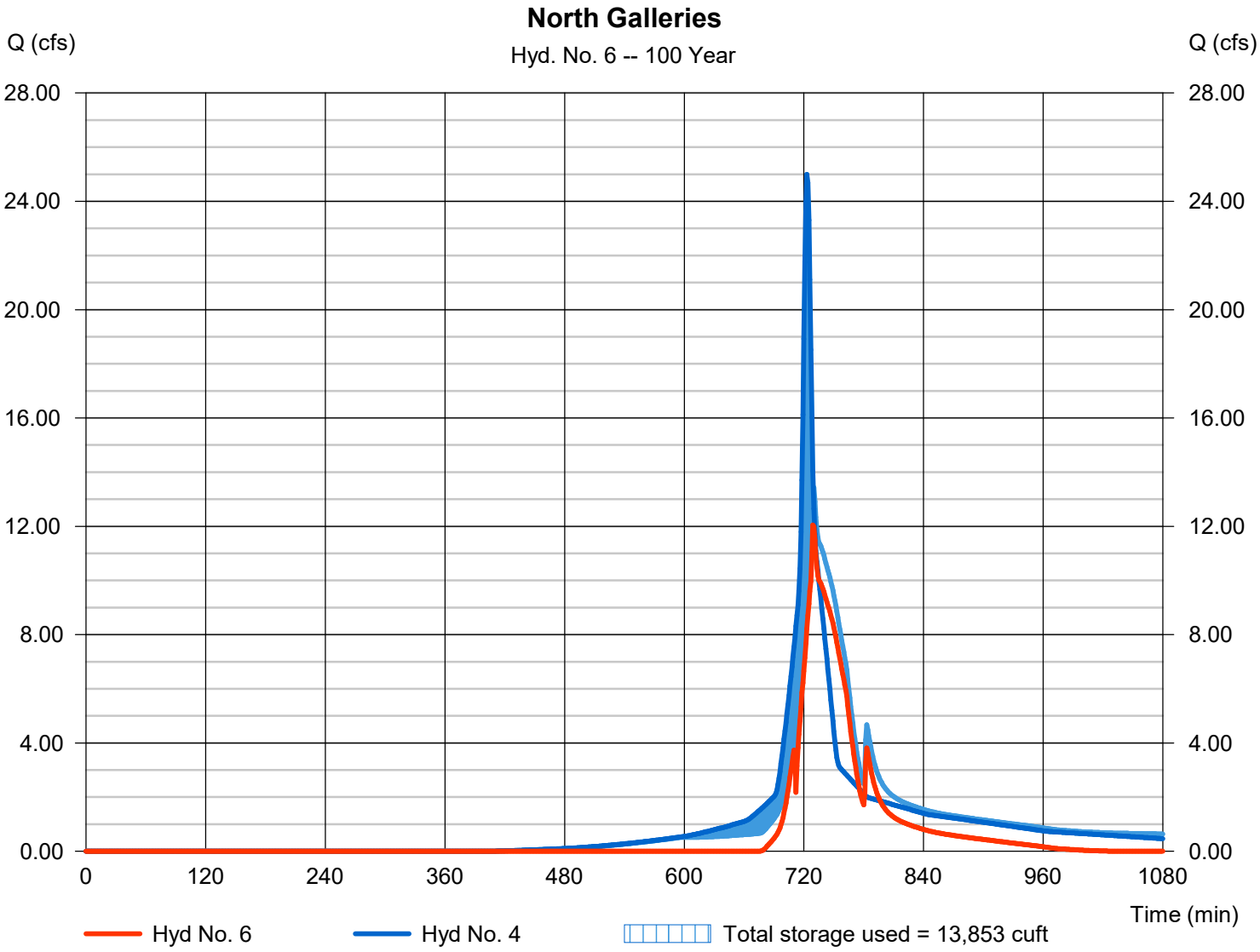
Hydrograph Report

Hyd. No. 6

North Galleries

Hydrograph type	=	Reservoir	Peak discharge	=	12.05 cfs
Storm frequency	=	100 yrs	Time to peak	=	729 min
Time interval	=	1 min	Hyd. volume	=	40,050 cuft
Inflow hyd. No.	=	4 - PROP DA-2	Max. Elevation	=	229.62 ft
Reservoir name	=	North Galleries	Max. Storage	=	13,853 cuft

Storage Indication method used. Exfiltration extracted from Outflow.



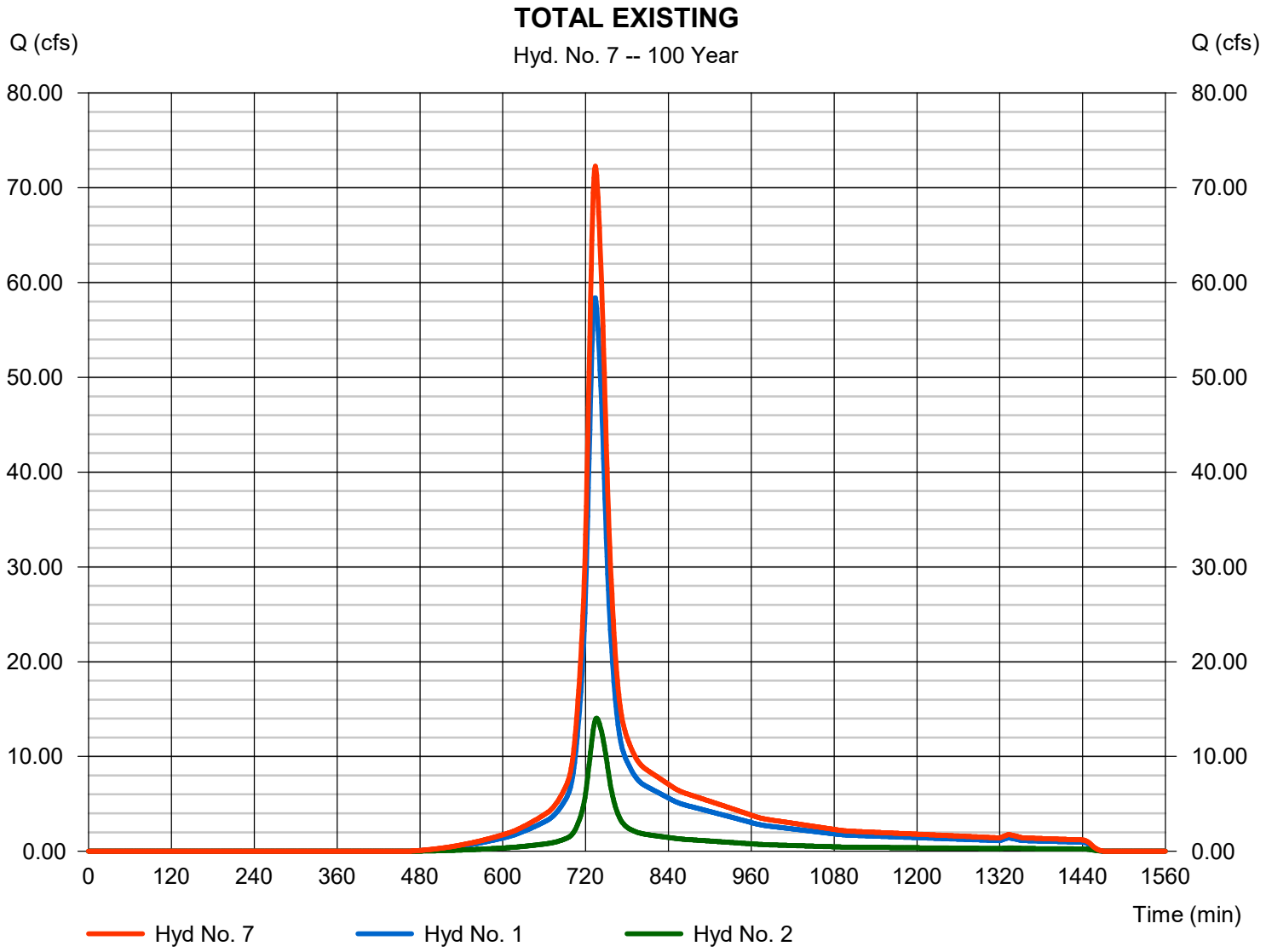
RECEIVED
By Marc J. Garofalo, MPA, MCC, MCTC at 10:45 am, Apr 14, 2026

Hydrograph Report

Hyd. No. 7

TOTAL EXISTING

Hydrograph type	=	Combine	Peak discharge	=	72.27 cfs
Storm frequency	=	100 yrs	Time to peak	=	734 min
Time interval	=	1 min	Hyd. volume	=	323,219 cuft
Inflow hyds.	=	1, 2	Contrib. drain. area	=	17.770 ac



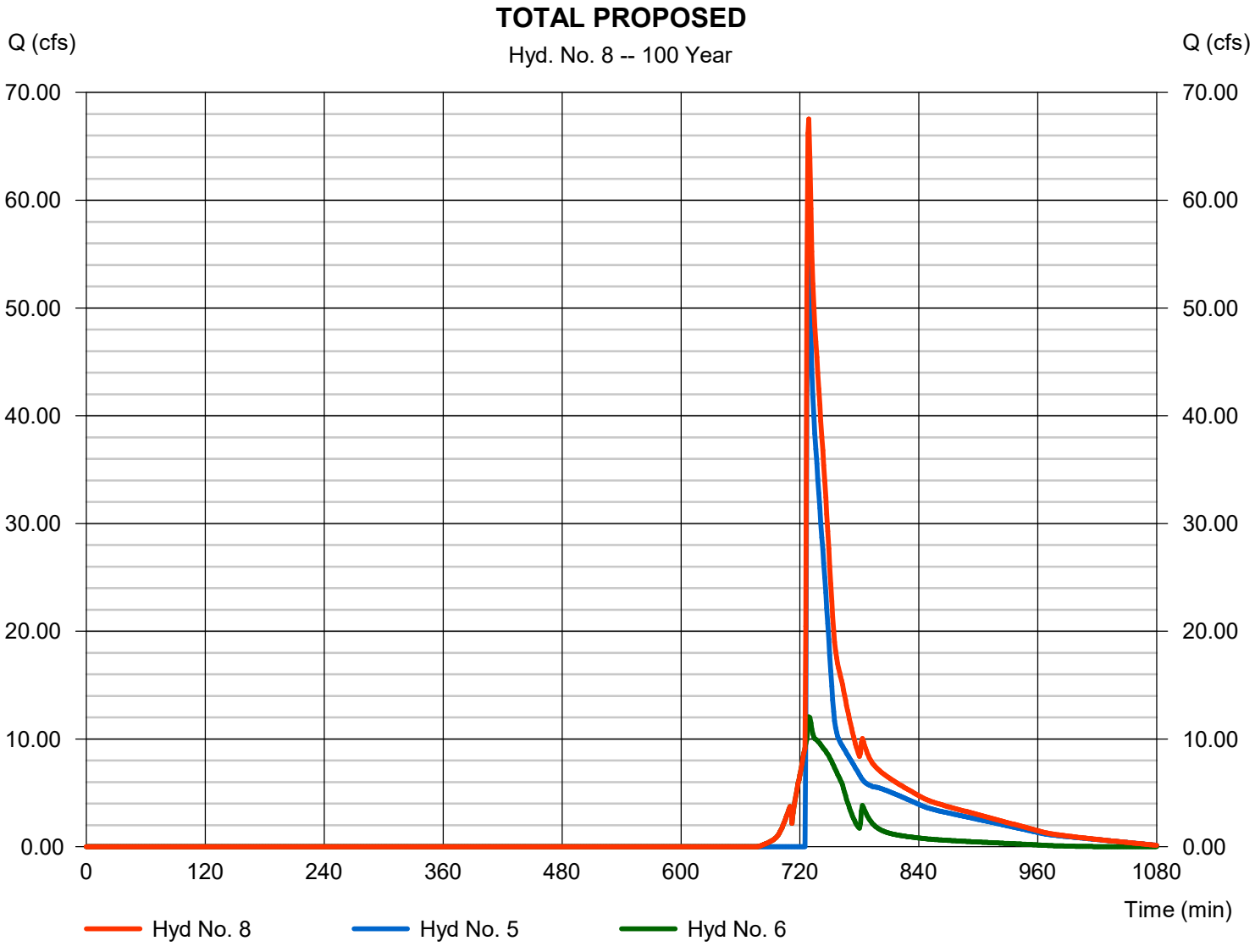
RECEIVED
By Marc J. Garofalo, MPA, MCC, MCTC at 10:45 am, Apr 14, 2026

Hydrograph Report

Hyd. No. 8

TOTAL PROPOSED

Hydrograph type	=	Combine	Peak discharge	=	67.54 cfs
Storm frequency	=	100 yrs	Time to peak	=	729 min
Time interval	=	1 min	Hyd. volume	=	148,791 cuft
Inflow hyds.	=	5, 6	Contrib. drain. area	=	0.000 ac



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Hydraflow Rainfall Report

Return Period (Yrs)	Intensity-Duration-Frequency Equation Coefficients (FHA)			
	B	D	E	(N/A)
1	0.0000	0.0000	0.0000	--------
2	23.3464	3.9000	0.7061	--------
3	0.0000	0.0000	0.0000	--------
5	29.3741	3.8000	0.7033	--------
10	32.9968	3.5000	0.6915	--------
25	40.1073	3.5000	0.6929	--------
50	46.2166	3.6000	0.6977	--------
100	51.9312	3.7000	0.6979	--------

File name: Milford.idf

Intensity = B / (Tc + D)^E

Return Period (Yrs)	Intensity Values (in/hr)											
	5 min	10	15	20	25	30	35	40	45	50	55	60
1	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
2	4.99	3.64	2.93	2.48	2.17	1.94	1.76	1.62	1.50	1.40	1.31	1.24
3	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
5	6.36	4.64	3.73	3.16	2.76	2.47	2.24	2.06	1.91	1.78	1.67	1.58
10	7.51	5.46	4.39	3.72	3.25	2.91	2.64	2.43	2.25	2.11	1.98	1.87
25	9.10	6.61	5.31	4.50	3.94	3.52	3.20	2.94	2.72	2.54	2.39	2.26
50	10.30	7.48	6.01	5.09	4.45	3.98	3.61	3.32	3.08	2.87	2.70	2.55
100	11.48	8.36	6.73	5.70	4.99	4.46	4.05	3.72	3.45	3.22	3.03	2.86

Tc = time in minutes. Values may exceed 60.

Precip. file name: H:\LIBRARY\DEPT\SITE\hydraflow\derby-noaa-ct.pcp

Storm Distribution	Rainfall Precipitation Table (in)							
	1-yr	2-yr	3-yr	5-yr	10-yr	25-yr	50-yr	100-yr
SCS 24-hour	3.00	3.66	0.00	4.75	5.64	6.88	7.80	8.78
SCS 6-Hr	2.04	2.42	0.00	3.05	3.57	4.29	4.83	5.40
Huff-1st	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Huff-2nd	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Huff-3rd	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Huff-4th	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Huff-Indy	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Custom	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00

RECEIVED
By Marc J. Garofalo, MPA, MCC, MCTC at 10:45 am, Apr 14, 2026

APPENDIX B

Drainage Area Maps

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By Marc J. Garofalo, MPA, MCC, MCTC at 10:45 am, Apr 14, 2026



DISCHARGE OF STORM WATER

STORM WATER AND ALL OTHER UNPOLLUTED DRAINAGE SHALL BE DISCHARGED TO SUCH SEWERS AS ARE SPECIFICALLY DESIGNATED AS COMBINED SEWERS OR STORM SEWERS OR TO A NATURAL OUTLET APPROVED BY THE SUPERINTENDENT AND THE STATE OF CONNECTICUT DEPARTMENT OF ENVIRONMENTAL PROTECTION. INDUSTRIAL COOLING WATER OR UNPOLLUTED PROCESS WATERS MAY BE DISCHARGED, ON APPROVAL OF THE SUPERINTENDENT AND THE STATE OF CONNECTICUT DEPARTMENT OF ENVIRONMENTAL PROTECTION, TO A STORM SEWER, COMBINED SEWER OR NATURAL OUTLET.

STORM DRAINS; CURBS AND GUTTERS

ALL STORM DRAINS DETERMINED BY THE CITY ENGINEER AND COMMISSIONER OF PUBLIC WORKS TO BE REQUIRED BY GOOD CONSTRUCTION PRACTICE SHALL BE CONSTRUCTED BY THE OWNER AT HIS SOLE EXPENSE. THE STORM DRAINS SHALL INCLUDE ALL NECESSARY PIPE LINES, CATCH BASINS, MANHOLES AND APPURTENANCES SUFFICIENT AND ADEQUATE TO PROVIDE PROPER DRAINAGE. GUTTERS AND CURBS SHALL BE PAVED WITH BITUMINOUS CONCRETE AT CATCH BASINS AND WHEREVER, IN THE OPINION OF THE CITY ENGINEER, THE GRADES WILL CAUSE EXCESSIVE EROSION.

LEGEND

- EXISTING EDGE OF PAVEMENT
- PROPOSED EDGE OF PAVEMENT
- PROPERTY LINE
- STREAM
- WETLANDS
- PROPOSED BUILDING
- 123.4 PROPOSED SPOT ELEVATION
- EXISTING 2' CONTOUR
- EXISTING 10' CONTOUR
- PROPOSED 2' CONTOUR
- EXISTING CATCH BASIN
- PROPOSED CATCH BASIN
- EXISTING MANHOLE
- PROPOSED MANHOLE
- EXISTING STORM PIPES
- PROPOSED STORM PIPES
- EXISTING SANITARY PIPES
- PROPOSED SANITARY PIPES
- B UNITS WITH BASEMENT
- 23 LOT NUMBER

GRAPHIC SCALE

40 0 20 40 80 160

(IN FEET)
1 inch = 40 ft.

ROSE •TISO & Co. LLC.
ARCHITECTS ● SURVEYORS ● ENGINEERS
WWW.ROSETISO.COM
35 BRENTWOOD AVENUE, FAIRFIELD, CT 06825
TEL: (203)610-6262 ● FAX: (203)610-6404

REVISIONS

NO.	BY	DATE	DESCRIPTION

PROJECT TITLE

PROPOSED SUBDIVISION

SUMMIT STREET EXTENSION DERBY, CONNECTICUT

Prepared For:

SUMMIT HILL, LLC

RECEIVED
By Marc J. Carrolino, MPA, MCCI, MCTC at 10:08 am, Apr 14, 2026

SHEET TITLE

EXISTING DRAINAGE AREA MAP

DESIGNED BY: SFS	SCALE: 1"=80'
DRAWN BY: SFS	DATE: 01-05-23
CHECKED BY: MJS	PROJECT NUMBER: 2508
CAD FILE: R:\2508\dwg\2508	

SEAL

STATE OF CONNECTICUT · MANUEL JOSE SILVA · 25011 · LICENSED PROFESSIONAL ENGINEER

SHEET NUMBER

C-1

TC PATH
MANNING'S VALUE = 0.011
PROPOSED 310' PIPE FLOW
AVERAGE SLOPE 6.2%

TC PATH
MANNING'S VALUE = 0.011
PROPOSED 164' SHEET FLOW
AVERAGE SLOPE 6.5%

TC PATH
MANNING'S VALUE = 0.15
PROPOSED 40' SHEET FLOW
AVERAGE SLOPE 8%

EX SUB AREA 2
3.63 AC CN=73

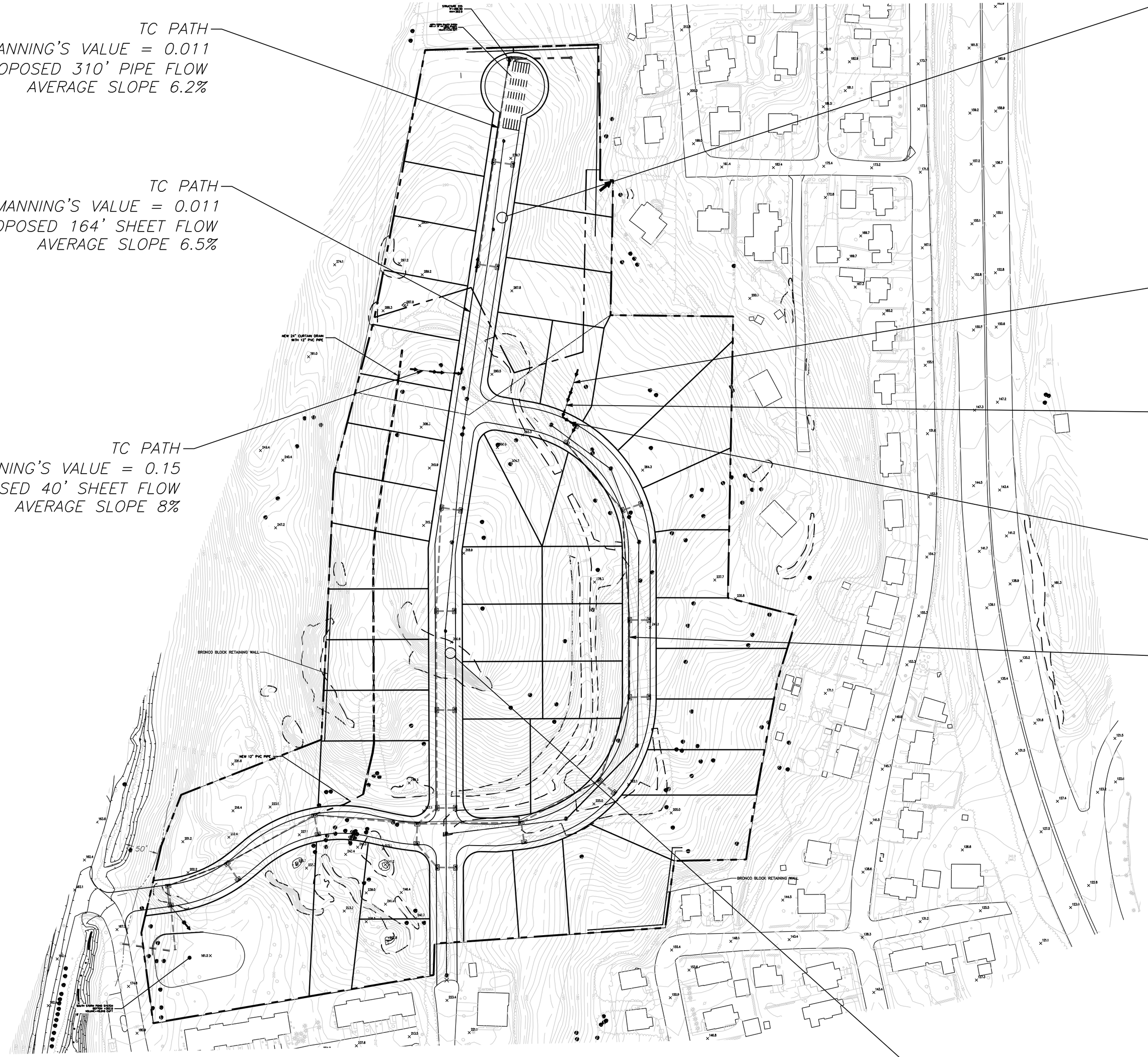
TC PATH
MANNING'S VALUE = 0.15
PROPOSED 40' SHEET FLOW
AVERAGE SLOPE 8%

TC PATH
MANNING'S VALUE = 0.15
PROPOSED 35' SHEET FLOW
AVERAGE SLOPE 5%

TC PATH
MANNING'S VALUE = 0.011
PROPOSED 24' SHEET FLOW
AVERAGE SLOPE 10%

TC PATH
MANNING'S VALUE = 0.011
PROPOSED 1,320' PIPE FLOW
AVERAGE SLOPE 8.0%

EX SUB AREA 1
14.14 AC CN=72



LEGEND

- EXISTING EDGE OF PAVEMENT
- PROPOSED EDGE OF PAVEMENT
- PROPERTY LINE
- STREAM
- WETLANDS
- PROPOSED BUILDING
- 1,2,3,4 PROPOSED SPOT ELEVATION
- EXISTING 2' CONTOUR
- EXISTING 10' CONTOUR
- PROPOSED 2' CONTOUR
- EXISTING CATCH BASIN
- PROPOSED CATCH BASIN
- EXISTING MANHOLE
- PROPOSED MANHOLE
- EXISTING STORM PIPES
- PROPOSED STORM PIPES
- EXISTING SANITARY PIPES
- PROPOSED SANITARY PIPES
- GRADE TO DRAIN
- SITE LIGHTING
- B UNITS WITH BASEMENT

GRAPHIC SCALE

40 0 20 40 80 160

(IN FEET)
1 inch = 40 ft.

ROSE •TISO & CO. LLC.
ARCHITECTS●SURVEYORS●ENGINEERS
WWW.ROSETISO.COM
35 BRENTWOOD AVENUE, FAIRFIELD, CT 06825
TEL: (203)610-6262●FAX: (203)610-6404

REVISIONS

NO.	BY	DATE	DESCRIPTION

PROJECT TITLE

PROPOSED SUBDIVISION

SUMMIT STREET EXTENSION
DERBY, CONNECTICUT

Prepared For:

SUMMIT HILL, LLC

SHEET TITLE

PROPOSED DRAINAGE AREA MAP

DESIGNED BY: SFS	SCALE: 1"=80'
DRAWN BY: SFS	DATE: 01-05-23
CHECKED BY: MJS	PROJECT NUMBER: 2508
CAD FILE: R:\2508\dwg\2508	

SEAL

STATE OF CONNECTICUT
MANUEL JOSE SILVA
No. 24924
LICENSED PROFESSIONAL ENGINEER

SHEET NUMBER

C-2